

Review

## Observational and numerical modeling methods for quantifying coastal ocean turbulence and mixing

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### Abstract

In this review paper, state-of-the-art observational and numerical modeling methods for small scale turbulence and mixing with applications to coastal oceans are presented in one context. Unresolved dynamics and remaining problems of field observations and numerical simulations are reviewed on the basis of the approach that modern process-oriented studies should be based on both observations and models. First of all, the basic dynamics of surface and bottom boundary layers as well as intermediate stratified regimes including the interaction of turbulence and internal waves are briefly discussed. Then, an overview is given on just established or recently emerging mechanical, acoustic and optical observational techniques. Microstructure shear probes although developed already in the 1970s have only recently become reliable commercial products. Specifically under surface waves turbulence measurements are difficult due to the necessary decomposition of waves and turbulence. The methods to apply Acoustic Doppler Current Profilers (ADCPs) for estimations of Reynolds stresses, turbulence kinetic energy and dissipation rates are under further development. Finally, applications of well-established turbulence resolving particle image velocimetry (PIV) to the dynamics of the bottom boundary layer are presented. As counterpart to the field methods the state-of-the-art in numerical modeling in coastal seas is presented. This includes the

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application of the Large Eddy Simulation (LES) method to shallow water Langmuir Circulation (LC) and to stratified flow over a topographic obstacle. Furthermore, statistical turbulence closure methods as well as empirical turbulence parameterizations and their applicability to coastal ocean turbulence and mixing are discussed. Specific problems related to the combined wave–current bottom boundary layer are discussed. Finally, two coastal modeling sensitivity studies are presented as applications, a two-dimensional study of upwelling and downwelling and a three-dimensional study for a marginal sea scenario (Baltic Sea). It is concluded that the discussed methods need further refinements specifically to account for the complex dynamics associated with the presence of surface and internal waves.

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## 1. Introduction

Turbulence is an important mechanism for transporting heat, salt, momentum and suspended and dissolved matter in the coastal zone. The so-called turbulent fluxes of material occur as a result of correlated, small-scale fluctuations in current velocity and the transported quantity itself. Increasing turbulence usually leads to increased turbulent fluxes as measured, for example, by the variance of the current velocity (a measure for the turbulent kinetic energy (TKE)) and the variance of the transported quantity. The horizontal and vertical turbulent fluxes are typically of the same order of magnitude. Due to the small aspect ratio (the ratio of vertical to horizontal length scales) in natural waters, horizontal advective fluxes caused by the mean flow dominate horizontal turbulent fluxes. Horizontal dispersion that has the characteristics of horizontal mixing results from shear dispersion, the joint effect of vertical mixing and differential horizontal advection due to current shear.

Turbulence is produced either by mean shear (loss of mean-flow kinetic energy) or by unstable stratification (loss of potential energy). An external source of turbulence is breaking surface waves which inject turbulence into the water column. In the coastal zone, mean shear is typically generated by winds or tides, but also by surface waves (Stokes drift) and baroclinic flows, including nonlinear internal waves. Unstable stratification results from surface processes such as surface cooling, evaporation or freezing and also as a result of differential advection. In the latter, vertically well-mixed waters become unstably stratified through current shear in situations such as upwelling zones, tidal estuaries and coastal areas during flood. Destruction of turbulence occurs by transformation into potential energy (during stable stratification) or viscous dissipation into heat.

Because of their statistical character, turbulent fluxes are difficult to observe, and must usually be quantified by indirect strategies. Often, observers measure turbulent statistics such as the vorticity variance (proportional to the dissipation rate of TKE), the Reynolds stress or the temperature variance. With the aid of simplifying assumptions (for example, the concept of eddy viscosity), measured quantities become estimators for the vertical turbulent fluxes. Numerical models incorporate parameterizations for quantifying turbulent fluxes. The quality of these parameterizations is tested by their ability to reproduce certain observable turbulent behavior.

It is the intention of this paper to give an overview of relevant processes, observational techniques and modeling strategies. The first two sections generally introduce the dynamics of the surface mixed layer (Section 2.1) and the bottom boundary layer (Section 2.2). The complex interaction between turbulence and internal waves is then discussed in Section 2.3. The following sections cover observational techniques and their methods of statistical data interpretation, in particular, micro-structure shear-probe profilers (Section 3.1), measurement under breaking surface waves (Section 3.2), near-bed Acoustic Doppler Current Profilers (ADCPs) (Section 3.3) and particle image velocimetry (PIV) (Section 3.4). In the modeling part, the method of Large Eddy Simulation (LES) is introduced in Section 4.1, followed by statistical models (Section 4.2) and empirical models (Section 4.3). Specific attention is given to the modeling of the wave–current bottom boundary layer, which is of high relevance in the coastal ocean, and combines both statistical and empirical approaches (Section 4.4). The paper concludes with a comparative study of two- and three-dimensional model results obtained from several empirical and statistical turbulence models (Section 4.5).

The dynamics of the coastal ocean is determined by a number of non-dimensional parameters and scaling laws, some of which are explicitly discussed here. A far more comprehensive overview is given in the textbook of [Kantha and Clayson \(2000\)](#) in the Appendix C.

For the notation of velocity vector components, we define  $(u, v, w) = (u_1, u_2, u_3)$  which mostly denote the eastward, the northward and the upward components, respectively.

## 2. Turbulent regimes

### 2.1. Surface boundary layer

The ocean surface boundary layer is the part of the ocean that is directly exposed to the atmosphere, through which atmospheric properties are transmitted into the ocean interior. It is the layer of the ocean in which there are high light levels, and therefore the potential for high biological productivity. Near-surface

turbulence creates a mixed layer immediately below the surface, in which quantities such as temperature and dissolved and particulate material are relatively uniform in the vertical.

Because of surface waves, there are significant differences between the ocean's surface mixed layer and the atmospheric boundary layer (ABL). The surface mixed layer is usually more complex than the ABL with turbulence often not clearly separable from other coherent structures.

On both sides of the air–sea interface, there are thin skin layers of a few millimeter thickness that are not turbulent, but controlled by molecular viscosity and diffusivity. Although the skin layer thickness on the water side is commonly only of the order of 1 mm it provides a significant bottle neck for many air–sea exchange processes. The skin sea surface temperature (SSST), which is typically about 0.3 K lower than the bulk SST, determines heat losses to the cooler atmosphere (by processes of latent and sensible heat flux and long-wave radiation, e.g. Taylor, 2000). The isolating skin layer is disrupted by breaking surface waves. At low to moderate wind speeds this occurs mainly in form of micro-breaking, that is, the breaking of very short wind waves without air entrainment, which plays an important role in air–sea gas exchange (Zappa et al., 2001). Larger breaking waves cause whitecapping, that injects air bubbles into the water (and water drops and spray into the air), so that the air–sea gas exchange increases tremendously at the location of breakers (e.g. Thorpe et al., 2003a).

At moderate to high wind speed, momentum is transferred from wind to ocean currents via wave breaking. The dissipation of wave energy occurs mainly in breaking waves. Thus, wave breaking is a source of enhanced TKE levels in the near-surface layer and plays an important role in upper ocean processes. Vertical transport of heat, gases and particles in the near-surface zone depends on turbulent transport. Generally, increased turbulence levels relate to enhanced air–sea exchange processes. Comprehensive overviews of the role of wave-induced turbulence in upper-ocean dynamics and air–sea exchange processes are given in a number of review articles (Thorpe, 1995; Melville, 1996; Duncan, 2001).

It is now accepted that energy dissipation rates (denoted by  $\varepsilon$ ) in the near-surface layer of a wind-driven ocean depart significantly from the classic wall-layer form, in terms of magnitude as well as depth dependence. There is general agreement that in a wind-driven sea the surface layer may be divided into three regimes. Wave breaking directly injects TKE into the top layer down to a depth  $z_b$ . In this injection layer, dissipation rates are highest and most probably independent of depth. Below  $z_b$ , the wave-induced turbulence diffuses downwards and dissipates. In this region, the turbulence decay rate is faster than the traditional wall-layer dependence  $\varepsilon \propto z^{-1}$ , which results from the assumption of a constant stress and zero surface flux of TKE, see also Eq. (10). At a depth  $z_t$ , sufficiently far from the air–sea interface, the contribution of waves becomes small compared with local shear production, and turbulence properties are well described by the wall-layer scaling. There is, as yet, no conclusive observational evidence for the vertical extension of the different regimes ( $z_b$ ,  $z_t$ ), nor the exact depth-dependence of the wave-induced turbulence. Suggestions for the latter include  $\varepsilon \propto z^n$ , with  $n$  in the range  $-4$  to  $-2$  (Terray et al., 1996; Drennan et al., 1996), and  $\varepsilon \propto e^{-z}$  (Anis and Moum, 1995). In the open ocean, under strong wind forcing in the presence of swell, Greenan et al. (2001) found enhanced dissipation rates but depth-dependence consistent with wall-layer scaling. The depth  $z_b$  of direct TKE injection determines the total dissipation rate, which in fully developed sea has to match the energy input from the wind (Gemmrich et al., 1994). Furthermore,  $z_b$  determines the surface mixing length  $z_0$ , a crucial parameter in turbulence closure models. The problem of properly observing  $z_b$  is related to the spatial reference frame for the measurements (see Section 3.2).

Other effects of surface waves are increased near-surface Reynolds stresses and related increases in horizontal transport due to Stokes drift. When the Stokes drift interacts with the wind-driven currents and the surface mixed layer is not too deep, Langmuir circulation (LC) is triggered, see Thorpe et al. (2003a) and Rascle et al. (2006). LC is visible as characteristic surface windrows that are caused by bubble clouds in horizontal, counter-rotating cells aligned in the wind direction. Since Langmuir cells are similar in size to large eddies, a strong interaction between turbulence and Langmuir cells may be observed by means of LES, leading to a phenomenon described as Langmuir turbulence (McWilliams et al., 1997 and also Section 4.1).

The major scaling parameter for turbulence in a wind-driven surface mixed layer is the surface friction velocity  $u_s^* = (\tau_s/\rho)^{1/2}$ , with the surface stress  $\tau_s$  and the density  $\rho$ . When positive buoyancy fluxes due to cooling and evaporation come into play, another parameter is of great importance, the so-called Deardorff velocity scale  $w^* = (B_0 D)^{1/3}$ , where  $B_0$  is the surface buoyancy flux and  $D$  is the mixed layer depth. When the ratio

$\Pi_* = u_{s*}/w_*$  falls below values of 0.65, the structure of the surface mixed layer becomes convectively dominated, with the typical counter-gradient fluxes of buoyancy (see the LES results by Moeng and Sullivan (1994) and Heitmann and Backhaus (2005)). The relevant non-dimensional quantity that determines the importance of buoyancy fluxes in establishing LC is the Hoenikker number  $H_O = B_0/(k \cdot u_{s*}^2 \cdot V_{SO})$ , with the wave number  $k$  and the Stokes velocity  $V_{SO}$ , see Li and Garrett (1995), Skillingstad and Denbo (1995) and Li et al. (2005) for details.

The lower boundary of the surface mixed layer is either the seabed (in case the water is sufficiently shallow) or a pycnocline established by vertical gradients of temperature and/or salinity. In case of a pycnocline, internal waves may be generated by various processes such as shear instability, large turbulent eddies, surface waves, spatial and temporal atmospheric forcing inhomogeneities, currents interacting with bottom topography. These internal waves will interact with turbulence (see Section 2.3).

In the case that the seabed is the lower boundary of the surface layer, the water column is completely mixed. This happens typically in tidally dominated coastal areas when the pycnocline is eroded from below. Simpson and Hunter (1974) suggest a non-dimensional parameter of the form  $(-HB_0/\bar{u}_s^3)$  (with depth  $H$ , surface buoyancy flux  $B_0$  and r.m.s. bed friction velocity  $\bar{u}_s$ ) as an estimator for whether the water column is mixed or stratified (see Simpson et al., 1977). As a rough and easily measurable indication for tidal front locations, Simpson and Hunter (1974) use the dimensional ratio  $H/\bar{u}_s^3$ , where  $\bar{u}_s$  denotes the surface velocity amplitude. When  $H/\bar{u}_s^3$  is in the order of a critical value of about 50–100 s<sup>3</sup>/m<sup>2</sup>, depending on the surface momentum and buoyancy fluxes, a frontal zone is expected nearby. Tidally induced frontal zones are known for complex mechanisms of cross-frontal circulation (see Dong et al., 2004) and high biological productivity (see Richardson et al., 1998).

In shallow coastal water, the influence of surface waves may penetrate right to the bed and create a wave bottom boundary layer that can interact with the mean tidal (pressure-gradient-driven) or stress-driven flow (see Section 4.4 and observational evidence in Section 3.4). In contrast to a stress-driven flow, in a pressure-gradient-driven flow, the Reynolds stress might have a maximum value at the bottom boundary (see observations of such well-mixed flows in Section 3.3).

## 2.2. Bottom boundary layer

In coastal regions, turbulent mixing near the marine bed is influenced by the interaction of current and wind-wave boundary layers and related sediment transport. A bottom boundary layer spans the vertical distance between the no-slip bottom boundary condition and outer, free-stream motion. The height of the boundary layer is related to the magnitude of the free-stream velocity, bottom roughness and the time available for boundary layer development. A wave boundary layer is typically much thinner, but far more turbulent, than a current boundary layer. When the two boundary layers co-exist, they interact in a transient, nonlinear manner, though the effect of currents on the wave boundary layer is generally much less than the effect of waves on the current boundary layer.

### 2.2.1. Steady and tidal flow

Classic near-bed flow follows the law of the wall, leading to a logarithmic velocity profile which is determined by the bed shear stress and a roughness length. (The roughness length is formally defined as the distance above the bed of the position at which the extrapolation of the logarithmic profile has zero velocity.) Such a typical wall layer will occur in stationary, unstratified and irrotational flows over plain, solid beds.

Natural bottom boundaries usually deviate significantly from the idealized picture of a wall layer. Beds are not plain, but have ripples and dunes of various length scales, which contribute to an additional so-called form drag. Seen from the body of the flow, ripples and dunes act as roughness elements in such a way that multiple logarithmic layers may be observed (Soulsby, 1983). In shallow coastal waters, the oscillatory effect of surface waves on the bottom boundary layer may also significantly increase the effective bed roughness as felt from the bulk of the flow (see Section 2.2.2). Moreover, the bed does usually not consist of a hard substrate but particulate sediments which may be suspended into the water column when bed stress is present. Thus, bed forms may be influenced by the flow, density stratification caused by the sediment itself may damp near-bed turbulence, and down-slope density currents may be generated (Section 2.2.3).

Slowly varying flow such as tidal flow exhibits some peculiarities. When the water column is accelerated from a situation in which the near-bed velocity is zero (as after slack tide), shear stress propagates upwards from the bed. Other turbulent properties such as shear production, TKE and dissipation rate also propagate upwards, all with a height dependent time lag behind the phase of the bed stress. This has been clearly demonstrated by means of analytical models (e.g. Prandle, 1982), while observational evidence of the phase lag of dissipation has been provided, for example, by Simpson et al. (2000). Recent independent observations of turbulence production and dissipation rate by Rippeth et al. (2003) in a narrow strait allow the study of the production-dissipation phase lag, which is often ignored in modeling exercises for boundary layer flow. The turbulent structure of tidal bottom boundary layers may also be strongly influenced by horizontal density gradients. Due to tidal straining, turbulence is enhanced when the flow is towards water of lower density, and vice versa, as in the observations of Rippeth et al. (2001) and the simulations of Simpson et al. (2002). A model simulation of this effect is presented in Section 4.2.4.

### 2.2.2. Wave–current bottom boundary layer

In shallow coastal waters, the orbital motion due to surface waves may create horizontal oscillatory currents in the near-bed region. Linear wave theory predicts significant near-bed orbital motions for water depths less than  $0.16gT_w^2$ , with  $T_w$  denoting the wave period, and  $g$  denoting the gravitational acceleration (see Souza, 2005). Due to the rapid oscillatory nature of wave-induced motion, a wave boundary layer is typically much smaller in height (a few centimeters) than a current boundary layer (meters to tens of meters). This leads to steeper velocity gradients and more intense turbulence in the wave boundary layer. When wave and current boundary layers co-exist, the overlying currents appear to perceive the wave boundary layer as an additional bed roughness contribution, leading to increased bottom friction and modified mean current profiles (Kemp and Simons, 1982, 1983). As a result, the mean bottom stress in a combined flow is greater than the sum of bottom stresses found in independent wave and current boundary layers.

Fig. 9 (Section 3.4) provides an example of the effect of waves on the mean current profile. For strong tidal currents dominating the motion near the bed, a classic logarithmic profile would be expected, with a straight-line portion that extends close to the bed. In contrast to that Fig. 9 shows how the effect of relatively weak tidal motion has led to a typical wave–current boundary layer profile, consistent with the form postulated by Grant and Madsen (1986). The straight-line portions end well above the bed and one can estimate the apparent bottom roughness by extending these lines to intercept the velocity axis.

In recent decades, various model approaches towards simulating the wave–current bottom boundary layers have been suggested. Among these, the empirical theory of Grant and Madsen (1986) and its extensions have received the greatest attention in the scientific community. An alternative statistical approach consistent with statistical turbulence closure models (Section 4.2) has recently been suggested by Mellor (2002). For a discussion of these models, see Section 4.4.

### 2.2.3. Effects of sediment

Sediment has three main effects on the structure of the bottom boundary layer: (i) variable bed forms, (ii) turbulence damping and (iii) generation of down-slope turbidity currents (for an overview see Friedrichs, 2005).

The bed forms (ripples and dunes) are generated by means of a complex interaction between near-bed currents and the seabed. For short-term investigations the impact of currents on bed forms may be neglected, but on longer time scales, this interaction may cause morphological modifications of coastal areas.

When the bottom shear stress of the current exceeds a certain critical value (which depends on a number of factors, including the bed shape, and sediment composition and compaction), sediment is eroded and resuspended into the water column. At high sediment concentrations, the density of the sediment-laden water may be such that significant stratification is generated, damping vertical turbulent mixing. For fine sediments with small settling velocities, a well-mixed sediment-laden layer may be generated, with a sharp vertical density gradient (a so-called lutocline, see Noh and Fernando, 1991) separating it from the non-turbid waters. If the sediment concentrations are high enough, the velocity profiles may deviate significantly from the law of the wall (see Friedrichs et al., 2000). The presence of a wave boundary layer will also enhance sediment entrainment (Glenn and Grant, 1987). Modeling of these effects is an area of current research (e.g. Winterwerp, 2001).



Downslope turbidity currents are generated when sediment is resuspended in a sloping bottom layer. For sufficiently steep slopes, these currents may be self-accelerating, increasing the resuspension rate with increasing speed which, in turn, increases the speed with increasing density and sediment layer height. For a recent overview over sediment gravity currents, see [Parsons et al. \(2007\)](#).

### 2.3. Internal waves and turbulence

In sufficiently deep shelf seas, well-mixed surface and bottom boundary layers may be separated by stable vertical density stratification. This stratification may be permanent, for example near river outflows, but is usually intermittent, varying for example on a seasonal scale. Vertical stratification inhibits vertical turbulent exchange, but, it also supports internal waves. As a result, interior turbulent mixing can be generated by shear associated with internal waves, or even by breaking internal waves. The relationship between internal waves and turbulence is subtle: they can be difficult to separate in observation, and difficult to parameterize in models. In modeling, both internal waves and turbulence tend to occur at subgrid scales, but it is through their generation of turbulence that internal waves tend to have impact on larger-scale dynamics.

In theory, turbulence and (internal) waves are easily distinguishable physical processes. Three-dimensional turbulence is inherently dissipative, that is, it causes mixing and net fluxes of heat, material and momentum. Freely propagating internal waves do not cause a net flux or transport of material, and they satisfy a dispersion relation, that is, a potentially observable relationship between frequency and wavelength. However, in practice, internal waves are not easily distinguished in oceanographic data because two-dimensional, quasi-geostrophic turbulence, in the form of small eddies, occurs at the same scales ([Holloway, 1983](#); [Müller, 1984](#)). Furthermore, net fluxes are extremely hard to measure directly. Turbulent fluxes may need to be inferred from additional information on, for example, environmental conditions like stratification and external (turbulence-generating) sources like wind stress.

Freely propagating internal gravity waves exist in the frequency ( $\sigma$ ) range  $f < \sigma < N$ ,  $N \gg f$ , where  $f$  is the effective local inertial frequency, due to both the rotation of the earth and low-frequency vorticity ([Mooers, 1975](#); [Kunze, 1985](#)), and

$$N(x, y, z, t) = \left( -g \frac{d \ln \rho}{dz} \right)^{0.5}, \quad (1)$$

the buoyancy frequency, is a measure of the background stratification. Both  $f$  and  $N$  vary on space and time scales larger than those of internal waves. In the open ocean, at intermediate frequencies  $f \ll \sigma \ll N$ , the kinetic energy  $E(\sigma)$  satisfies the classic Garrett–Munk spectrum,

$$E(\sigma) \propto N \sigma^{-2}, \quad (2)$$

([Garrett and Munk, 1972](#)). However, for current measurements from a single-point mooring, a smooth spectral continuum of this form does not immediately signal the presence of internal waves, because it is barely distinguishable from the spectrum for “fine structure contamination” ([Phillips, 1971](#)). Further, shelf seas do not show a spectral continuum like Eq. (2) but exhibit enhanced energy at frequencies like  $M_4 - f$ ,  $M_2 + f$ ,  $M_4 + f$ ,  $M_6 + f$ , ([van Haren et al., 1999](#)), signifying internal wave-generation by nonlinear interaction between tidal and wind-driven inertial motion ([Fig. 1](#)).

For current measurements, an independent method for distinguishing linear internal waves from turbulent eddies is to use rotational spectra ([Gonella, 1972](#); [van Haren, 2003](#)). Linear internal waves exhibit circular polarization near  $f$ , asymptoting smoothly to rectilinear motion near  $N$ .

As noted, internal waves generate turbulence in their own right. Observations by [Mack and Hebert \(1999\)](#) show that most mixing associated with internal waves is induced through shear instability. Current shear  $\mathcal{S}$  is defined by

$$\mathcal{S}(z, t) = \left( \frac{\partial u}{\partial z}, \frac{\partial v}{\partial z} \right) \quad (3)$$

and the competition between the stabilizing influence of stratification and the destabilizing effect of shear is expressed in the mean gradient Richardson number,

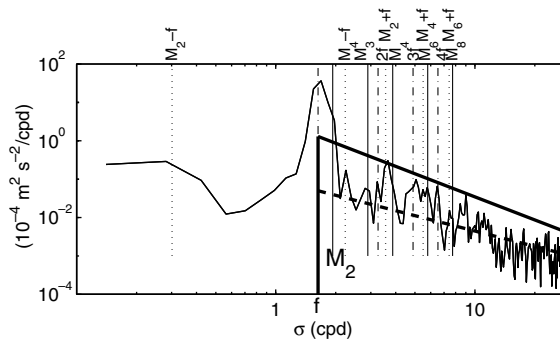


Fig. 1. Nearly unsmoothed ( $\sim 3$  degrees of freedom) kinetic energy spectrum for 8 days of baroclinic currents observed in strong summertime stratification in the central North Sea. Barotropic (tidal) currents are subtracted from the original record. Some frequencies are indicated (until  $M_8$ ). The sloping solid line corresponds with a fall-off rate of  $\sigma^{-2}$ , the dashed line indicates  $\sigma^{-1}$ .

$$Ri(x, y, z, t) = \frac{N^2}{|S|^2}, \quad (4)$$

Paradoxically, large static stabilization  $N$  is usually accompanied by large destabilizing vertical current shear. In the North Sea, a subtle balance was observed for moderately large  $N$ , with  $Ri$  close to constant and taking a value in the range 0.5–1 (Fig. 2; van Haren et al., 1999). Thermocline values of  $Ri \sim 1$  are also reported for the seasonally stratified Irish and Celtic Continental Shelf Seas (Rippeth, 2005; Rippeth et al., 2005).

A constant  $Ri$  of order 1 has also been confirmed in deep water, for example above the Bermuda slope (Eriksen, 1978) and in the abyssal ocean (Pinkel and Anderson, 1997), where values of  $S$  and  $N$  were two orders of magnitude lower than those in the North Sea. Eriksen (1978) suggested that observations similar to those in Fig. 2 were indicative of a balance between support of internal waves by stratification and their destruction by shear. This balance is described as “saturated internal waves”. The same value was also suggested for such saturation by Munk (1981).

For small  $N$  and for largest  $N$  and  $|S|$ ,  $Ri$  is observed to take a value of 0.25. For  $Ri < 0.25$ , linear analysis suggests that a transition to turbulence will occur (Miles, 1961; Howard, 1961). However, under a strongly nonlinear internal wave regime, a turbulent-transition value of  $Ri \approx 1$  has been suggested (Abarbanel et al., 1984; see also Miles, 1990, for a review of Richardson numbers and stability).

In the open ocean, shear is predominantly induced via large-scale internal wave (baroclinic) flows. By contrast, in stratified shelf seas, it is mostly a result of externally forced, barotropic, viscous flows (Maas and van Haren, 1987). The two major sources are tidal shear generated via friction at the bottom, and inertial shear

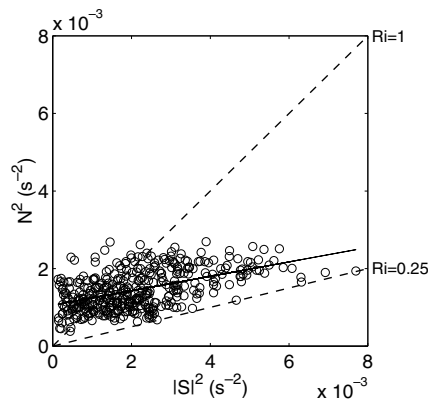


Fig. 2.  $Ri$ -statistics observed at the depth of strong summertime stratification in the central North Sea. At this depth the rms slope (thin solid line) of the distribution equals 0.18, with axis intercept at  $1.1 \times 10^{-3} \text{ s}^{-2}$ .



generated by wind forcing at the surface. Large-scale shear distorts small-scale (higher frequency) internal waves to the point of breaking, resulting in mixing. Several practical methods have been suggested to estimate such internal wave-induced mixing.

Gargett (1984) inferred from microstructure measurements that vertical turbulent diffusivity  $K_H$  scales with  $N$ ,

$$K_H \propto N^{-1}, \quad (5)$$

Thus, assuming that internal wave breaking dominates diapycnal exchange, Eq. (5) provides a simple way to estimate relative changes in mean turbulent diffusion. Given that  $N$  in shelf seas varies between  $10^{-3} \text{ s}^{-1}$  (in winter under strong mixing conditions) and  $10^{-1} \text{ s}^{-1}$  (under summer stratification of  $5^\circ \text{C m}^{-1}$ ),  $K_H \approx 10^{-3}$ – $10^{-5} \text{ m}^2 \text{ s}^{-1}$ , as observed in the North Sea (van Haren et al., 1999). Even though interior diapycnal exchange rates are weak in summer, they are nevertheless  $O(100)$  times larger than molecular diffusion.

Instead of Eq. (5), under the saturated wave condition of  $Ri \sim 1$  a more appropriate parameterization relating internal waves and diapycnal exchange is suggested (e.g. Pinkel, 1981) as

$$K_H \propto |S|^{-1}. \quad (6)$$

Shear  $S$  in Eq. (6) has the advantage of being directly measurable using ADCPs, which may also be able to provide simultaneous estimates of turbulent parameters (Section 3.3).

For accurate prediction both of water properties and of material in the water, parameterizations of diapycnal mixing due to internal waves need incorporation in numerical models. Until quite recently, internal waves and their impact on diapycnal mixing were not or could not be modeled properly. Essentially, shelf-scale numerical models used grid-spacing too coarse to resolve internal wave scales. On the other hand, specific numerical internal wave studies generally used idealized, detailed models, mainly of internal tides (e.g. Xing and Davies, 1998; Gerkema, 2001). Such models provided important information on internal tide generation and propagation, but did not incorporate dissipation through wave breaking. Fortunately, renewed interest in internal waves and its relevance for mixing and the large scale circulation in the ocean (Munk and Wunsch, 1998), has led to a new generation of models that incorporate nonlinear internal waves (e.g. Hibiya et al., 1998; Xing and Davies, 2002; Furue, 2003) and internal wave parameterization in terms of energy fluxes (St. Laurent and Garrett, 2002). Using the canonical relation for the mechanical energy budget of turbulence (Osborn, 1980), St. Laurent and Garrett (2002) parameterized turbulent diffusivity as

$$K_H = \frac{\Gamma \xi E(x, y) F(z)}{\rho N^2}, \quad (7)$$

where  $\Gamma \approx 0.2$  denotes a mixing efficiency,  $\xi \approx 0.3$  dissipation efficiency,  $E(x, y)$  the time-averaged map of generated internal tide energy flux and  $F(z)$  the function for the vertical structure of dissipation. Thus far,  $E$  and  $F$  are provided from (separate) numerical models for tidal waves only, and observational input is still lacking as Eq. (7) is more complex than Eqs. (5) and (6) for practical use of ocean measurements.

### 3. Observation and data interpretation

To measure turbulence, the relevant fluctuating physical properties must be recorded in a highly-resolved, four-dimensional domain (three space and one time dimension). Further, the measurements must be of sufficient intensity and duration to yield robust statistical properties such as the TKE or the turbulent energy dissipation into heat. In the ocean however, such observations are generally not possible and, particularly, do not usually cover more than two dimensions. Typical observations are as follows:

1. time-series at a fixed point (such as acoustic Doppler velocimeter measurements from a bottom mounted rig);
2. time-series from towed or freely sinking (or rising) or rig-mounted profilers;
3. repeated profiles from acoustic profilers such as the ADCP; and
4. repeated snapshots made by PIV in a two-dimensional plane.

Methods 1 and 2 cover a one-dimensional space (time, or space by interpretation as “frozen turbulence”), method 3 accommodates two dimensions (space and time) and method 4 a three-dimensional space (two-dimensional space and time). All methods have in common that they miss at least one spatial dimension.

Turbulence is highly intermittent, so that spatial or temporal under sampling is always an issue. Furthermore, none of the methods above is able to cover the full range of spatial scales from the smallest dissipative eddies (typically of the order of a centimeter) to the largest energy-containing eddies (typically of the order of several meters). Thus, to quantify statistics of turbulence, additional assumptions have to be made. The extrapolation to the four-dimensional space usually assumes local isotropy (that is, the turbulence has no preferred direction), at least on the smallest scales. To overcome the problem of only covering a part of the turbulence spectrum, it is assumed that larger scales do not significantly contribute to the micro-structure turbulence (for observations of the dissipation of TKE into heat) and that the smaller scales do not contribute to the energy-containing scales (for observations of Reynolds stress and kinetic energy). Furthermore, Taylor’s frozen turbulence hypothesis is often invoked, using an instrument’s profiling speed to transform time-series of turbulent fluctuations into vertical profiles. These assumptions are all only partly justified: turbulence observations never have the accuracy achievable for mean properties such as temperature or currents.

In the following sections, three different turbulence observation techniques are presented, beginning with the mechanical profiling micro-structure shear probes (Section 3.1). Section 3.2 is dedicated to the specific problems of observing turbulence beneath breaking surface waves. Then, a novel acoustic profiling method is presented (Section 3.3), followed by optical PIV (Section 3.4). For all methods, the specific data analysis problems are discussed.

### 3.1. *Micro-structure shear-probe observations*

A microstructure shear probe has a radially symmetric airfoil faced in the profiling direction. While the probe is not sensitive to axial forces, the cross-stream (transverse) components of turbulent velocity produce a lifting force at the airfoil. A piezoceramic beam, connected to the airfoil, senses the lift force. The output of the piezoceramic element is a voltage proportional to the instantaneous cross-stream component of the velocity field. Lueck et al. (2002) provide an historical review of shear-probe construction and use, while Prandke et al. (2000) give full details on the processing of probe data.

#### 3.1.1. *Problems, limitations and measurement strategy*

Generally, the accuracy of shear measurements cannot be compared with that of standard oceanographic parameters (temperature, electrical conductivity). There are several sources of errors in shear measurements and consequently, in the estimation of dissipation rates, including limitations in the mathematical assumptions, determination of profiling velocity, errors in the calibration procedure, drift of the shear-probe sensitivity, temperature and pressure effects and interference from profiler vibrations. Intercomparison of two microstructure profilers equipped with shear sensors, or comparison of dissipation rates based on shear measurements and temperature microstructure measurements have shown an overall accuracy of dissipation measurements within a factor of two (Moum et al., 1995; Kocsis et al., 1999). Recent improvements of the long-term stability of shear sensors (Wolk et al., 2002), profiler constructions and data processing schemes (Prandke et al., 2000) promise even better accuracy.

The intensity of turbulence and turbulent mixing in the ocean is highly intermittent in amplitude, space and time. The degree of intermittency can be estimated by the intermittency factor, defined as the variance of the (natural) logarithm of the dissipation rate. Analysis of various data sets from stratified stations have shown an approximately lognormal distribution of the dissipation rate with an intermittency factor in the range 3–7 (Baker and Gibson, 1987). With such large intermittency, determination of mean dissipation rates requires a large number of statistically independent measurements. Consequently, turbulence measurements in the ocean are mostly carried out in the form of time series (repeated measurements at a fixed station). Baker and Gibson (1987) estimated a number of 2600 or 10000 independent data samples to obtain a mean dissipation rate with  $\pm 10\%$  accuracy at the 95% confidence level in stratified water with an intermittency factor of 3 or 7, respectively.

### 3.1.2. Example of measurements

Dissipation rate measurements have been carried out in October 1998 in the northern North Sea in the framework of the European Commission Marine Science and Technology Program project PROVESS. The dissipation rate calculation is based on shear measurements from two microstructure profilers. The FLY profiler, operated by the University of Wales, Bangor, is equipped with Osborn/Oakey shear sensors (Osborn, 1974; Oakey, 1977). The MSS profiler, operated by the Joint Research Center Space Applications Institute, is equipped with PNS shear sensors (Prandke, 1994). The measurements have been carried out simultaneously from two research vessels (*Pelagia* and *Dana*, approximately 1 nm apart) during a 20 h period. Sixty-five profiles have been taken with the MSS and 72 profiles with the FLY over the depth range 10–100 m (Prandke et al., 2000).

The temporal evolution of the dissipation rate obtained from the two profilers is shown in Fig. 3. It is obvious that the overall structure of the measured dissipation rate field agrees between the profilers. The high dissipation rates above  $10^{-7}$  W/kg in the top layer are caused by strong wind up to 15 m/s during most of the measurements period. The increase of the dissipation rate below 90 m is caused by bottom friction. Differences in the temporal evolution of the dissipation rates can be seen in the detail of the measurements. No profiler differs from the other in a systematic way and the observed differences can be assumed to be mainly due to the intermittency of the turbulence in the measurement region. The general scheme of the distribution of the dissipation rates from the microstructure profilers was validated by numerical models (Burchard et al., 2002).

### 3.2. Observing turbulence beneath breaking waves

According to Kolmogorov's hypothesis, there exists an inertial subrange where the three-dimensional velocity wavenumber spectrum of a high Reynolds number flow has a universal form that depends only on the energy dissipation. Assuming isotropic turbulence, Kolmogorov's hypothesis may be extended to measurements along a single direction and the one-dimensional wavenumber spectrum  $E_1(k)$  takes following form (Hinze, 1975):

$$E_1(k) = A \frac{18}{55} \varepsilon^{2/3} k^{-5/3}, \quad (8)$$

where  $k$  is the wavenumber and  $A = 1.5$  is a universal constant. This simple relationship between energy dissipation and wavenumber spectra allows the estimation of  $\varepsilon$  from velocity (rather than the velocity shear)

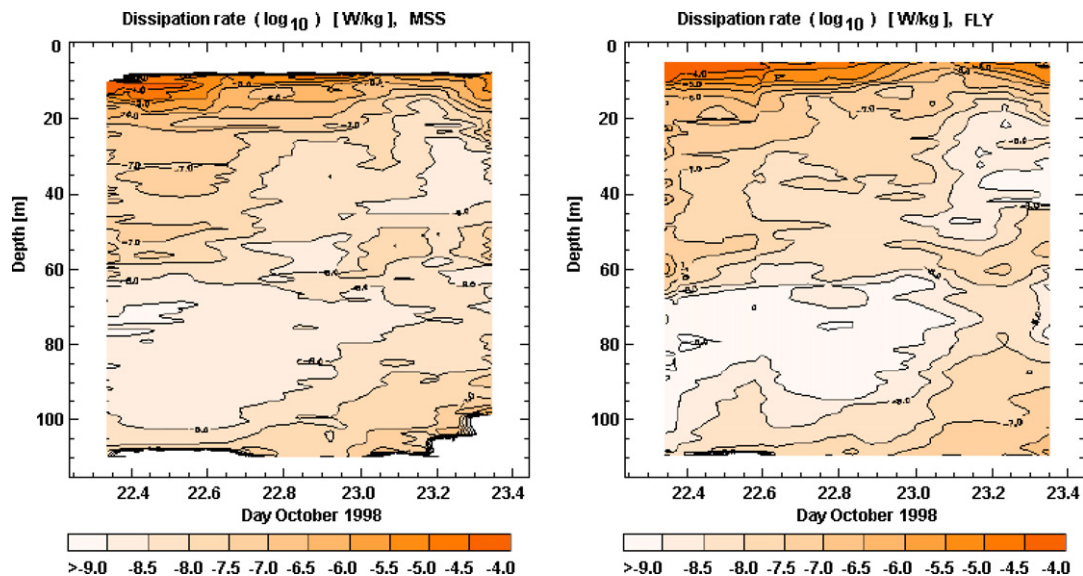


Fig. 3. Temporal evolution of the dissipation rates measured by MSS (left) and FLY (right) during the PROVESS measurements campaign in October 1998 in the northern North Sea. The figure has been taken from Prandke et al. (2000).

measurements. With recent advances in sonar technology it is now possible to resolve instantaneous velocity profiles at spatial ( $O(10^{-2} \text{ m})$ ) and temporal ( $O(10^{-1} \text{ s})$ ) scales suitable for turbulence measurements (Veron and Melville, 1999; Gemmrich and Farmer, 2004). All previous turbulence studies are based on single point velocity measurements. These time series yield frequency spectra which have to be converted into the wavenumber domain via Taylor's frozen turbulence hypothesis. Measurements from a fixed mooring (Agrawal et al., 1992; Terray et al., 1996) rely on the extension of Taylor's hypothesis for unsteady advection by the wave orbital motion (Lumley and Terray, 1983). Towed or vessel-mounted velocity measurements (Stewart and Grant, 1962; Drennan et al., 1996; Soloviev et al., 1999) minimize the additional uncertainties associated with unsteady advection.

Surface waves are a source of enhanced dissipation rates. They also create major difficulties for near-surface velocity measurements. Typical turbulent velocity fluctuations are  $O(10^{-2}–10^{-1} \text{ m s}^{-1})$ , 10–100 times smaller than the wave related velocities. The wave motion contains two to four orders of magnitude higher kinetic energy levels than the turbulent motion. Further, the nonlinear advection associated with the wave orbital motion modulates the turbulent flow observed at a fixed mooring and will certainly affect the dissipation rate estimates obtained from single point velocity records. A better separation between wave and turbulent flows can be achieved in the wavenumber domain. The spatial scale of turbulence is generally much smaller than the spatial scale of velocity fluctuations associated with surface waves. Therefore, wave-induced velocity fluctuations are not a significant problem in profile measurements with sonars. Fast moving single point velocity meters, e.g. bow measurements from a moving ship (Stewart and Grant, 1962; Soloviev et al., 1999) or mounted on submersibles (Osborn et al., 1992) also avoid the separation problems found in fixed-point measurements.

The oscillation of the sea surface poses a further challenge for observation and interpretation of near-surface turbulence. Mooring or tower-based observations (Kitaigorodskii et al., 1983; Agrawal et al., 1992) are limited to observations below the troughs, and depth is referenced to the mean still water line. Surface following measurements from floats (Gemmrich and Farmer, 1999a, 2004) or ship based (Stewart and Grant, 1962; Drennan et al., 1996; Soloviev and Lukas, 2003) are, in principle, able to monitor the region above the troughs, in which case depth is referenced to the instantaneous surface. For illustration, let us assume observations at 1 m depth. In the Agrawal et al. (1992) tower based observations in Lake Ontario at significant wave height  $H_s = 0.5 \text{ m}$  the equivalent surface referenced depth would be 0.75–1.25 m. At the other extreme, the Gemmrich and Farmer (1999a, 2004) float measurements in the open ocean at  $H_s = 3 \text{ m}$  would be recorded as depth ranging from  $-0.5 \text{ m}$  to  $+2.5 \text{ m}$  in a fixed coordinate system. Considering the strong depth dependence of dissipation rates (discussed below), the choice of coordinate systems will determine the resultant dissipation rate profile.

Microstructure profilers (Soloviev et al., 1988; Anis and Moum, 1992, 1995) operated in a rising mode are capable of observing turbulence up to the sea surface. Enhanced dissipation rate levels, assumed to be associated with wave breaking, are very intermittent, and unless the turnaround time of the profiler is short compared to the time between high dissipation events, these events are not resolved properly. At moderate to high wind speeds, typical intervals between breaking events are 20–100 s (Gemmrich and Farmer, 1999b) and are much shorter than the profiling repetition intervals.

Despite the observational challenges, quality turbulence data in the aquatic near-surface layer in the presence of breaking waves are accumulating (Stewart and Grant, 1962; Kitaigorodskii et al., 1983; Soloviev et al., 1988; Agrawal et al., 1992; Osborn et al., 1992; Anis and Moum, 1992, 1995; Drennan et al., 1996; Gemmrich and Farmer, 1999b, 2004; Greenan et al., 2001; Soloviev and Lukas, 2003). The majority of these data sets indicate that there are significant differences in the wind-driven oceanic boundary layer (OBL) compared to the ABL over land. Whereas turbulence properties in the ABL may be approximated by the constant stress layer scaling (also called law of the all scaling), this is not true for the OBL.

Measurements by Kitaigorodskii et al. (1983) from a tower in Lake Ontario were among the first to reveal significant dissipation rate enhancement in the near-surface layer. The reported three order of magnitude enhancement seems anomalously high relative to more recent measurements. The general consensus points at

$$\varepsilon_n = \varepsilon / \varepsilon_{\text{wall}} \cong O(10), \quad (9)$$

where  $\varepsilon$  is averaged over several minutes, and

$$\varepsilon_{\text{wall}} = u_*^3 / (\kappa z), \quad (10)$$

is the dissipation rate in the law of the wall scaling, with von Karman's constant  $\kappa \approx 0.4$ ,  $z$  the distance from the surface, and  $u^*$  the surface friction velocity. Recent observations by Gemmrich and Farmer (2004) reveal the specific contribution of wave breaking. The distribution of dissipation rate enhancement based on 1 s averages consists of a broad distribution in the range  $-0.5 < \log(\epsilon_n) < 1.5$  and a narrow log-normal distribution centered at  $\log(\epsilon_n) \sim 2$  (Fig. 4). The latter has been attributed to active breaking waves, whereas the lower enhancement rates are associated with periods between breaking events. At longer averaging periods, e.g. 1 min, the individual breaking events are no longer resolved, the maximum enhancement decreases to  $\log(\epsilon_n) \leq 2$  and the peak of the distribution shifts to  $\log(\epsilon_n) \sim 1$ .

Gemmrich and Farmer (1999a) monitored the temperature fine structure in the upper 2 m. Based on the direct measurement of vertical extension of temperature fluctuations they determined the surface mixing length  $z_0 = 0.2$  m. This is in good agreement with the depth of direct air entrainment  $z \sim 0.2$  m found in their conductivity measurements. Soloviev and Lukas (2003) infer similar  $z_0$  values from fitting the Craig and Banner (1994) model to dissipation rate data obtained with a bow-mounted microstructure sensor system. For the month-long experiment, with significant wave heights ranging from 1 m to 2.5 m, the best fit was obtained with  $z_0 = 0.1H_s$ . Much larger values were suggested by Terray et al. (1996) who were the first to argue that the surface mixing length should scale with the significant wave height. Matching model prediction to observations, they required  $z_0 = H_s$ .

It is interesting to note that Terray et al. (1996) based their finding mainly on fixed coordinate measurements, whereas Gemmrich and Farmer (1999a) and Soloviev and Lukas (2003) used surface-following measurements. Since more than half of the energy is dissipated above the mean water line (Stewart and Grant, 1962), it is more appropriate to use a wave-following coordinate system for the analysis of near-surface turbulence measurements.

Near-surface turbulence observations seem to be consistently indicating a 1–2 order-of-magnitude dissipation rate enhancement due to the effect of wave breaking. However, there is as yet no consensus on the depth-dependence and the mixing length. Further experimental evidence will be needed to resolve these two issues. The most promising observational approach might be the use of high-resolution acoustic Doppler profilers. As discussed in the following section, these instruments are capable of providing profiles of turbulent parameters. There is also a need to better sample the region above the trough line, implying the use of wave-following coordinates.

### 3.3. Near-bed high-resolution ADCP data

In the past decade, ADCPs have become central to the study of both the biological and physical oceanography of shelf seas. The ADCP exploits the Doppler effect by transmitting sound at a fixed frequency and

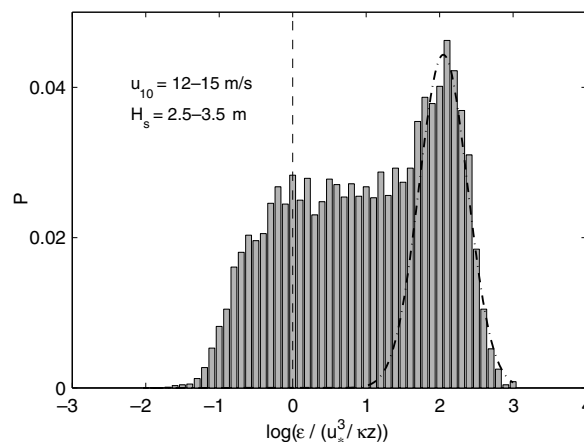


Fig. 4. Distribution of normalized near-surface dissipation rates, at 1 Hz sampling, underneath waves. Data were obtained from surface-following vertical sonar measurements at 1–1.7 m depth from the free surface and significant wave heights between 2.5 m and 3.5 m Gemmrich and Farmer (2004).  $P$  is here the normalized probability.



listening to echoes returning from sound scatterers in the water column. The along-beam velocity of the scatterers (which are assumed to move with the water) is calculated from the Doppler shift of the echo. An ADCP normally has four beams inclined at 20° or 30° to the vertical. The Cartesian velocity components ( $u, v, w$ ) are then calculated from the along-beam velocities.

More recently the availability of higher-frequency, broad-band ADCP has enabled the estimation of turbulence parameters. Gargett (1999) developed a method for estimating the rate of dissipation of TKE from measurements of larger-scale turbulent structures using an ADCP with one beam oriented in the vertical.

### 3.3.1. The variance method

The technique for estimating Reynolds stress is known as the variance method and relies upon the comparison of the velocity variances of opposing beams. The along-beam velocities ( $b_i, i = 1, 4$ ) are separated into a mean,  $\bar{b}_i$ , and a fluctuating quantity,  $b'_i$ , as

$$b_i = \bar{b}_i + b'_i. \quad (11)$$

If beams 1 and 2 are assumed to be in the  $x$ - $z$  plane, and the instrument is sitting flat on the seabed, the along-beam variances are given by Lu and Lueck (1999) and Stacey et al. (1999),

$$\begin{aligned} \overline{b_1^2} &= \overline{u^2} \sin^2 \theta + 2\overline{u'w'} \sin \theta \cos \theta + \overline{w^2} \cos^2 \theta, \\ \overline{b_2^2} &= \overline{u^2} \sin^2 \theta - 2\overline{u'w'} \sin \theta \cos \theta + \overline{w^2} \cos^2 \theta, \end{aligned} \quad (12)$$

where  $u'$  and  $w'$  are the fluctuations in the Cartesian velocity components and  $\theta$  is the inclination of the beams to the vertical. Taking the difference of the variances yields the  $x$ -component of the Reynolds stress  $\tau_x$  while the analogous relations for beams 3 and 4 yields  $\tau_y$ ,

$$\frac{\tau_x}{\rho} = \overline{u'w'} = \frac{\overline{b_1^2} - \overline{b_2^2}}{2 \sin 2\theta}, \quad (13)$$

$$\frac{\tau_y}{\rho} = \overline{v'w'} = \frac{\overline{b_3^2} - \overline{b_4^2}}{2 \sin 2\theta}. \quad (14)$$

Conversely, by taking the sums of the variances in Eq. (12), we have two quantities related to the TKE which are independent of the covariances,

$$\begin{aligned} d_u^2 &= \frac{1}{2}(\overline{b_1^2} + \overline{b_2^2}) = \overline{u^2} \sin^2 \theta + \overline{w^2} \cos^2 \theta, \\ d_v^2 &= \frac{1}{2}(\overline{b_3^2} + \overline{b_4^2}) = \overline{v^2} \sin^2 \theta + \overline{w^2} \cos^2 \theta. \end{aligned} \quad (15)$$

In order to determine the total TKE,  $q^2/2$  from  $d_u$  and  $d_v$ , it is necessary to have another independent estimate of one component, for example,  $w'$  measured using a fifth ADCP beam. Alternatively,  $w'$  can be estimated by assuming an anisotropy ratio,  $\alpha = w'^2/(u'^2 + v'^2)$ . However, this approach is not particularly satisfactory since the difference in  $\alpha$  between fully anisotropic and fully isotropic turbulence results in a sixfold difference in the value of  $q^2/2$  for an ADCP with  $\theta = 20^\circ$  (Lohrmann et al., 1990).

As Eqs. (13) and (14) are in the form of the difference of variance, the noise contribution will cancel out to the first-order, provided the noise characteristics of all the beams are well matched. There is, however, a sampling error (Stacey, 1999; Stacey et al., 1999) which may be reduced by averaging over time or increasing the size of the “bins”, the depth-interval over which the measurements are averaged (Rippeth et al., 2003).

In making the compromise between accuracy and resolution it is important to ensure that the spatial resolution of the system is sufficient to resolve the eddy scales which are responsible for the eddy transfer. Simpson et al. (2005) suggest that the vertical bin size should be set at less than 1.2 times the height above the bed, in order to capture the decreasing eddy scale as the bed is approached. Two further conditions should also be satisfied for the application of the variance method: stationarity and horizontal uniformity. The former sets the limit of the averaging period, and for regions dominated by the semidiurnal tide is usually taken as 10 or 20 min. Horizontal uniformity of the turbulence is required across the scale of the ADCP beam spread.



Problems also arise if the instrument alignment is not normal to the seabed, as the instrument tilt will bias the stress estimate, although the bias is small provided the tilt angles  $<6^\circ$  (e.g. Lu and Lueck, 1999; Rippeth et al., 2003). A potentially bigger problem is the presence of surface waves, which significantly bias the stress estimates even for small ( $\sim 2^\circ$ ) misalignments in the instrument mounting (Rippeth et al., 2003).

As an example application of the variance method, we shall consider recent observations taken in the Menai Strait, UK (Rippeth et al., 2002), using a 1.2 MHz ADCP mounted on the seabed in a mean water depth of 15 m. The Menai Strait is a 20 km long channel, 300–800 m wide, which connects the Caernarfon Bay and Liverpool Bay areas of the Irish Sea. The strait is subject to strong tidal currents, of up to  $2.5 \text{ m s}^{-1}$  at springs, which ensure that the water column is almost always well mixed in the vertical. Fresh-water input to the channel is small so that the along channel density gradients are generally weak.

The ADCP, working with a ping rate of 2 Hz, was set up to record ensemble averages of along beam velocities over 2 s (i.e. 4 pings) and with a vertical bin size of 0.5 m. To account for the reduction in velocity variance from the 4-bin average, and for loss of stress due to eddies smaller than the bin size, a correction of 1.3 was applied to the stress estimates. This correction is based on a short data set in which every ping was recorded and a bin size of 0.25 m used.

A sample of the along-channel stress profiles together with the corresponding current profiles is given in Fig. 5. The stress generally decreases more or less linearly from maximum values near the seabed. The

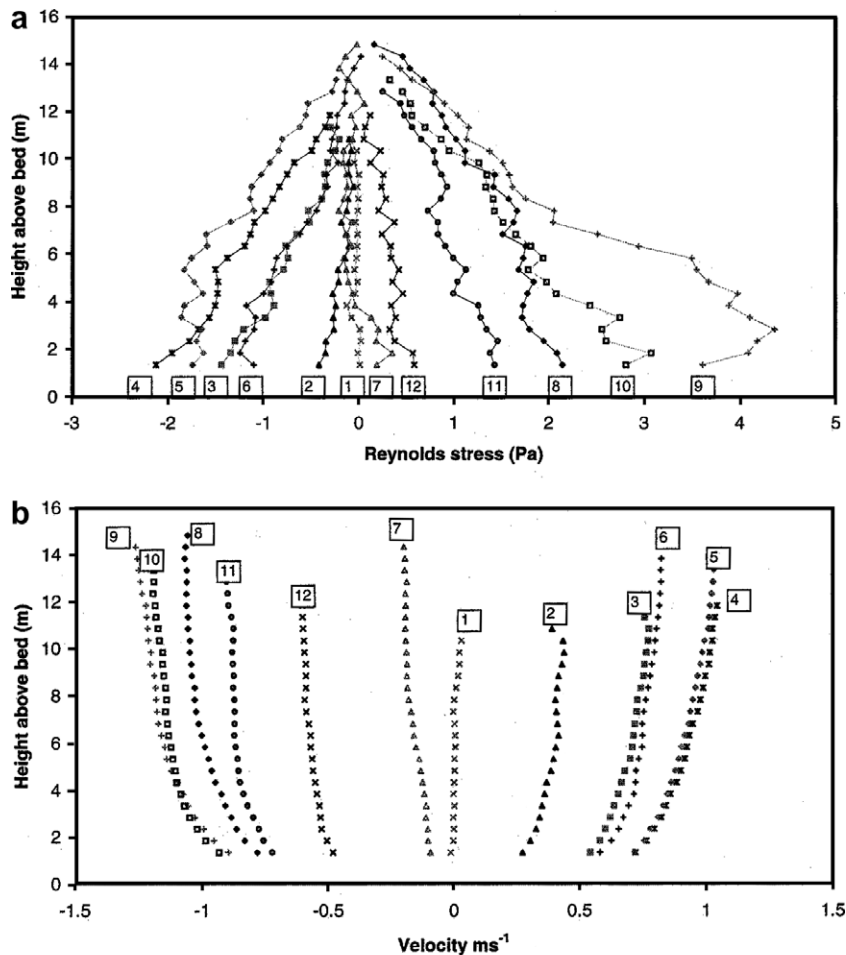


Fig. 5. Hourly (a) Reynolds stress (Pa) and (b) velocity profiles ( $\text{m s}^{-1}$ ) over a single tidal cycle during a deployment in the Menai Strait during a springs tide, in numerical sequence.

largest stress magnitudes occur at times of highest flow speeds with considerable asymmetry between the ebb and flood.

The stress estimates may be combined with the observed vertical shear to calculate the rate of production of TKE using

$$P = -\left(\tau_x \frac{\partial u}{\partial z} + \tau_y \frac{\partial v}{\partial z}\right). \quad (16)$$

Estimates of  $P$  made using the variance method have recently been compared to  $\varepsilon$  profiles made using a FLY microstructure profiler (see Section 3.1) in a homogeneous water column (Rippeth et al., 2003). In the absence of density stratification, dissipation should balance production except for a phase delay. The two parameters are found to track each other (Fig. 6) over the tidal cycle. The production estimates are higher than the dissipation estimates by a factor of 1.6, a result since attributed to inadequacy of the shear-probe response-function at high frequencies (Macoun and Lueck, 2004). On the basis of these measurements Rippeth et al. (2003) suggest that the lower threshold for estimation of  $P$ , using this setup for the ADCP, is about  $10^{-4} \text{ W m}^{-3}$ , several orders of magnitude above the lower limit of the FLY profiler. The introduction of an ADCP higher ping rate has enabled an improvement in this figure of up to an order of magnitude (Williams and Simpson, 2004).

More recently techniques have been developed for the estimation of  $\varepsilon$  using off-the-shelf acoustic Doppler profilers. Wiles et al. (2006) adapt a structure function technique developed for radar meteorology, to measure  $\varepsilon$  in the atmosphere to the marine environment. The structure function method uses the turbulent cascade theory of Kolmogorov to infer a spatial decorrelation trend of velocities that is proportional to the TKE dissipation rate. Lorke and Wüest (2005) use an inertial dissipation technique which uses a temporal spectral approach, to estimate  $\varepsilon$  in a low energy limnic environment.

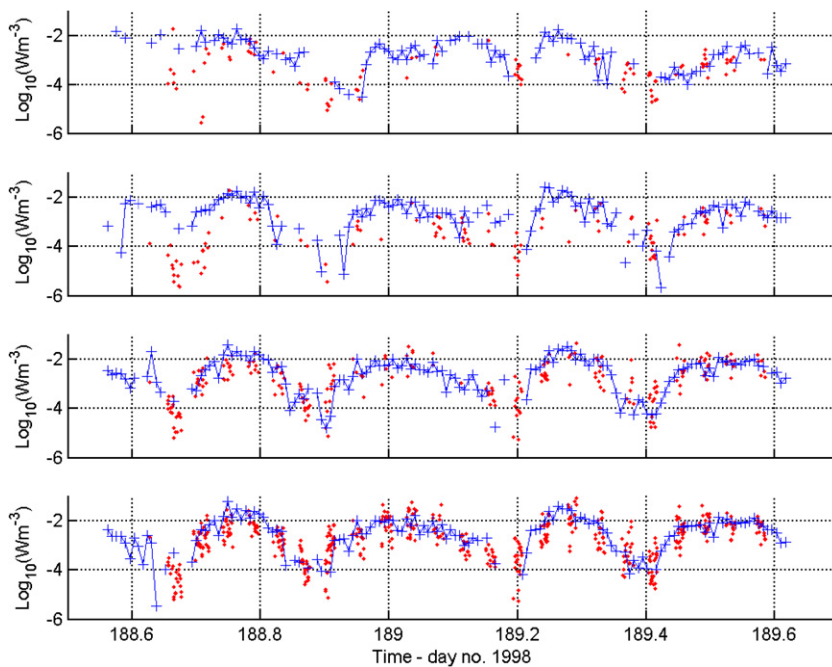


Fig. 6. A time series, covering two tidal cycles, of the rate of production (blue crosses) and dissipation (red dots) of TKE at heights of (a) 15.5 mab, (b) 10.5 mab, (c) 6.5 mab and 3.5 mab. Breaks in the  $P$  times series are indicative of negative  $P$  estimates. These measurements were made at a site in Red Wharf Bay, to the east of Anglesey, UK. The water depth  $\sim 25$  m, with rectilinear tidal currents of amplitude  $\sim 0.8$  m/s. At the time of the measurements, in July 1988, the water column was well mixed. Full details are reported in Rippeth et al. (2003).

### 3.4. Particle image velocimetry in the BBL

Particle image velocimetry (PIV) is a technique capable of mapping two components of the instantaneous velocity distribution within a section of a flow field (e.g. review by Adrian (1991)). PIV is achieved by seeding the flow with microscopic tracer particles and illuminating a thin 2D plane with a laser sheet. Pulsing the laser more than once, with a known interval between pulses, while recording an image of the 2D plane leaves multiple traces of each particle on the image. A velocity distribution is then extracted from each image by measuring the mean displacement of particles within small sub-windows of the image using an autocorrelation routine.

Unlike other instruments used for measuring the velocity distribution in the ocean, PIV provides an instantaneous two-dimensional velocity distribution over the entire sample area. A sequence of measurements provides a time series of the spatial distribution, not just a time series of velocity at a single point. With such data, it is possible to obtain an overview of entire flow structures and compute the vorticity distribution, rates of strain, turbulent stresses, turbulent spectra, and spatial and temporal correlations all without assuming Taylor's hypothesis. Many repeated measurements provide the turbulence statistics. Here, we show the application of a method to calculate the Reynolds stress in the presence of surface wave motions to the PIV data.

#### 3.4.1. Instrumentation

A schematic of the submerged components of an oceanic PIV system is shown in Fig. 7. The system is described fully in Nimmo Smith et al. (2002, 2004). It features two  $35 \times 35 \text{ cm}^2$  sample areas, imaged using high-resolution cameras, a mass data acquisition system and an extended-range profiling platform. The light source is a dual-head, dye laser that generates pairs of  $2 \mu\text{s}$  duration pulses. The laser is located on the support ship, and the light is transmitted through optical fibres to two submerged probes, containing the light-sheet forming optics that illuminate each of the sample areas. Images are acquired using two cameras, each capable of sampling at up to four frames per second. Naturally occurring particles are used as tracers.

The submersible components of the PIV system are mounted on a stable seabed platform, which can be rotated to align the sample areas with the mean flow direction, and extend vertically to sample the flow at different elevations up to 9.75 m above the seabed.

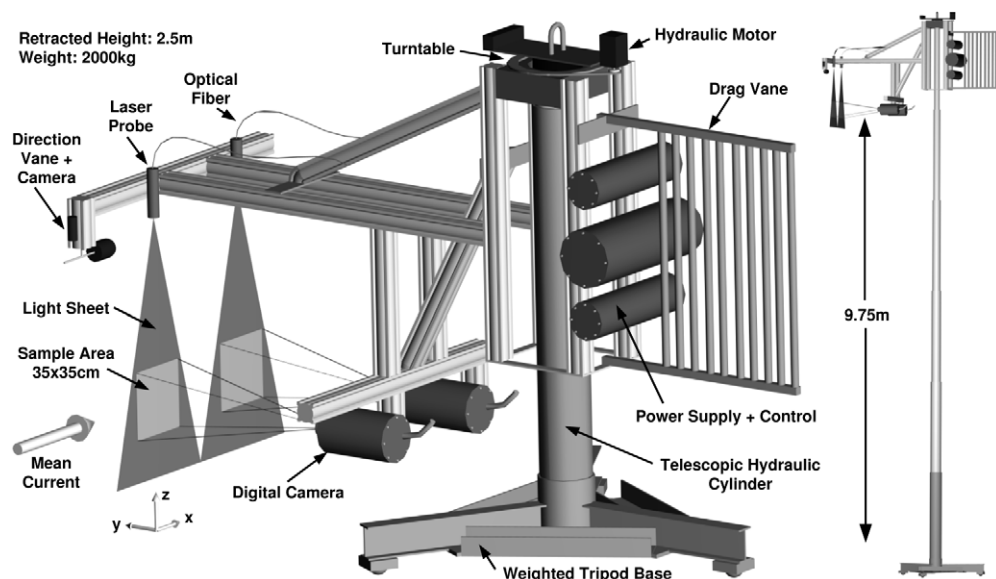


Fig. 7. Schematic of the submersible components of the ocean PIV system. On deployment the system is lowered to the seabed, extended to the desired sampling elevation and then rotated to align the sample areas to the mean current direction. The platform is shown fully extended in the inset figure to the right (from Nimmo Smith et al., 2005).

### 3.4.2. Example data

Deployments of the ocean PIV system have been performed several times at a number of locations off the East coast of the United States. Here, we show example data, collected near to the LEO-15 site off the coast of New Jersey in 15–21 m deep water, reported in full by Nimmo Smith et al. (2002, 2005). At this site, the sandy seabed is flat with some small bedforms. The area experiences near-bed tidal flows of up to 0.25 ms, and is exposed to the oceanic swell. Data were recorded at different elevations above the seabed for periods of 20 or 30 min, sampling at 2 or 3.33 Hz.

Fig. 8 shows two example instantaneous velocity distributions sampled about 0.5 m above the seabed, recorded 1 s apart under moderate flow conditions. The average instantaneous mean velocity has been subtracted from each vector to highlight the turbulent structures in this flow. Large (10 cm diameter) vortices are visible. These are advected by the combined action of the mean flow and oscillatory wave motion. They persist for periods in excess of 10 s, the time taken for them to be advected across both sample areas. In this flow, the vortices appear either singly, or in intermittently occurring groups, interspersed by periods of more quiescent flow. At periods of slack water, there are no large vortical structures present. To examine the effects that these vortical structures have on the statistics of the flow, we can conditionally sample the time-series of velocity maps based on the map-mean vorticity magnitude.

Fig. 9 shows sample vertical distributions of the time-series mean horizontal velocities,  $U(z)$ . For strong currents a logarithmic profile close to the seabed would be expected. However at LEO-15, even when the current is moderately strong and regardless of the phase of the wave motion, there is no evidence of a logarithmic profile (see Section 2.2.2). This is consistent with laboratory tests (Jensen et al., 1989), and most probably indicates the presence of a thick wave boundary layer.

### 3.4.3. Estimating the Reynolds stress

To calculate the Reynolds shear stresses, we follow the procedures introduced by Trowbridge (1998), which overcome problems introduced by instrument misalignment to any unsteady flow caused by surface waves. Briefly, the velocity is decomposed to  $u_i = \bar{u}_i + \tilde{u}_i + u'_i$ , where  $\bar{u}_i$  is the time average,  $\tilde{u}_i$  is wave-induced motion and  $u'_i$  is the turbulence contribution. We define the difference in instantaneous velocity at two points separated by a distance,  $r$ , as  $\Delta u_i = u_i(x_i + r_i) - u_i(x_i)$ . Assuming horizontal homogeneity, decomposing  $u_i$ , and assuming that there is no correlation between  $\tilde{u}_i$  and,  $u'_i$  one obtains

$$\text{cov}[\Delta u_i, \Delta u_j] = 2[\overline{\tilde{u}_i \tilde{u}_j} + \overline{u'_i u'_j}] - 2[\overline{\tilde{u}_i(x_i + r_i) \tilde{u}_j(x_i)} + \overline{u'_i(x_i + r_i) u'_j(x_j)}]. \quad (17)$$

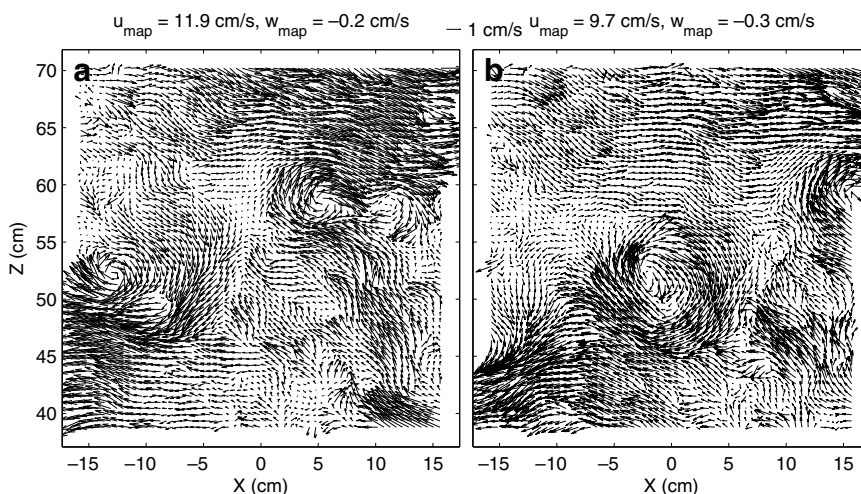


Fig. 8. Two example velocity vector maps of the same sample area, sampled 1 s apart. The instantaneous mean velocity of the sample area (shown at the top of each map) is subtracted from each vector to highlight the turbulence structure. The vertical coordinates represent the actual distance from the seabed. Large (10 cm diameter) coherent vortices can be observed as they are advected through the sample volume by the combined mean flow and oscillatory wave motion.

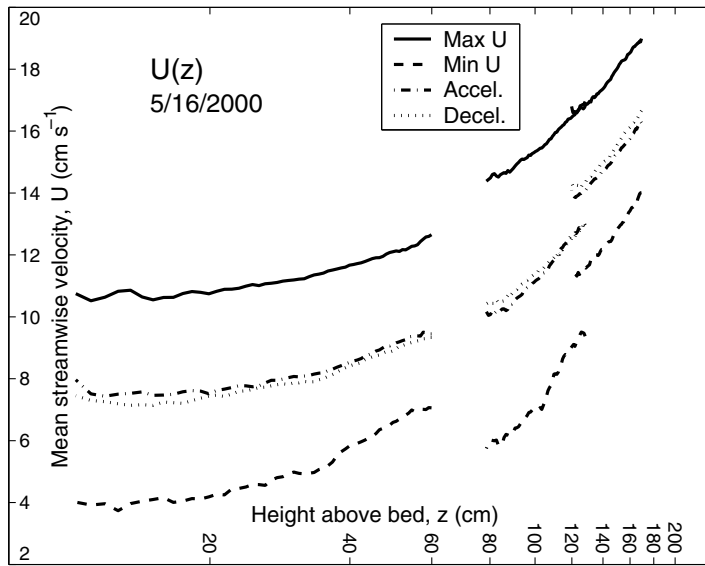


Fig. 9. Conditionally sampled vertical distributions of mean horizontal velocity at different phases of the surface-wave induced motion. The data shown is from three consecutive 30 min periods, one at each of the three elevations above the bed. The amplitude of the oscillatory wave motion is similar to the magnitude of the mean flow (from Nimmo Smith et al., 2002).

In flows with integral scale,  $l$ , much smaller than the wavelength of surface waves,  $\lambda$ , as long as  $r \ll \lambda$ ,

$$\text{cov}[\Delta u_i, \Delta u_j] = 2[\overline{u'_i u'_j}] - 2[\overline{u'_i(x_i + r_i) u'_j(x_j)}]. \quad (18)$$

The first term on the right-hand side is the Reynolds stress and the second term is the structure function,  $R_{ij}(r_i)$ . Typically,  $R_{ij}(r_i)$  decreases with increasing  $r$  and diminishes to zero when  $r$  is larger than the integral scale. Thus, for  $r \ll \lambda$ ,

$$\text{cov}[\Delta u_i, \Delta u_j] = 2[\overline{u'_i u'_j}], \quad (19)$$

i.e. the stress is equal to half the covariance of the velocity differences.

In this manner, one can calculate  $\text{cov}[\Delta u, \Delta w](r_1)$ , where  $r_1$  is the streamwise separation, using data obtained in a vertical plane. Sample distributions of  $\text{cov}[\Delta u, \Delta w](r_1)$ , calculated from velocity maps that have been conditionally sampled based on the map-mean vorticity magnitude, are presented in Fig. 10. The values of  $\text{cov}[\Delta u, \Delta w]$  increase with  $r_1$ , but converge to constant values as  $r_1$  becomes comparable to the integral scale. Performing the calculation across both sample areas would allow for larger integral scales. The condition of  $r \ll \lambda$  is also satisfied ( $\lambda \approx 100$  m). The conditional sampling clearly shows  $\overline{u'w'}$  that is substantially larger during periods of high vorticity, when the flow contains large vortical structures (Nimmo Smith et al., 2005). Using this approach, PIV data of a vertical plane can be used for obtaining the vertical distribution of  $\overline{u'w'}$  (Luznik et al., 2007). In the future, stereo PIV could be used to provide the vertical distributions of all three shear stress components,  $\overline{u'w'}$ ,  $\overline{u'v'}$  and  $\overline{v'w'}$ .

#### 4. Modeling

There are various strategies for modeling turbulence, each of them resolving a different interval of the spectrum of the flow dynamics:

- *Direct Numerical Simulation (DNS)*: Here, the Navier–Stokes equations which describe all scales of the flow from viscous to global are directly solved by means of discretization techniques. Since all scales in the ocean are interacting, the viscous scale always needs to be resolved by this technique, and thus, computational restrictions limit the method to small scales at relatively low Reynolds numbers (see e.g. Shih



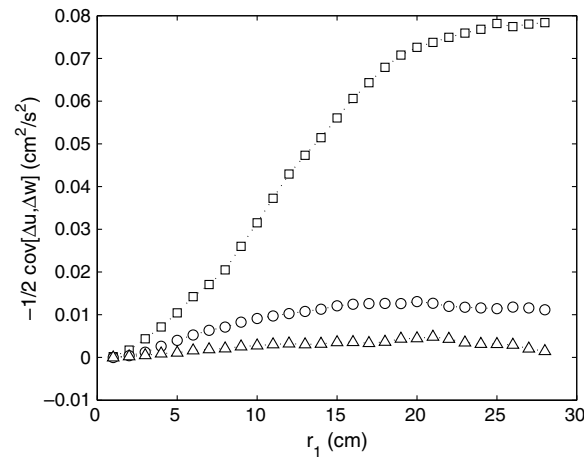


Fig. 10. Sample distributions of  $\text{cov}[\Delta u, \Delta w](r_1)$  using velocity maps from one of the PIV sample areas. The three distributions are from the same 20 min sampling period but calculated from velocity maps that have been conditionally sampled based on the map-mean vorticity magnitude. Squares: high vorticity (see Fig. 8); circles: intermediate vorticity, triangles: low vorticity.

et al., 2000, for homogeneous shear flows and Smyth et al. (2001), for single Kelvin–Helmholtz billows). Therefore, DNS cannot be applied to specific coastal scale problems and is not further discussed in the present review. The interested reader is referred to the excellent text book by Ferziger and Perić (2001).

- *Large Eddy Simulation (LES)*: This technique solves the spatially filtered Navier–Stokes equations, such that the large, energy-containing turbulent eddies are resolved while the micro-scale is parameterized (see Ferziger and Perić (2001), for methodological details). As with DNS, LES is not suitable for simulations on larger scales. In oceanography, this method is usually applied for investigating near-surface mixed layers, mostly with horizontally homogeneous macroscopic conditions, see e.g. Wang et al. (1998) and Large and Gent (1999) for the equatorial Pacific, Jones and Marshall (1993) and Denbo and Skillingstad (1996) for oceanic convection and Skillingstad and Denbo (1995) and McWilliams et al. (1997) for Langmuir circulation. For the coastal ocean, LES is a relatively new field. Two applications are discussed in Section 4.1.
- *Statistical turbulence modeling*: By contrast with LES, these techniques solve Reynolds-averaged Navier–Stokes (RANS) equations with ensemble means rather than fluctuating quantities as prognostic variables. Thus, the random character of turbulence is represented with a statistical method. For applications in the ocean, the RANS equations are usually simplified by various local equilibrium assumptions and closed by means of parameterizations. For further details, see Section 4.2.
- *Empirical turbulence modeling*: This type of model is usually based on energy-conservation considerations in the oceanic mixed layer, and traditionally predicts integral properties such as mixed-layer depth, mixed-layer temperature and salinity, and entrainment or detrainment velocity (Kraus and Turner, 1967; Denman, 1973). More recent realizations also resolve the vertical structure of the mixed layer by relating vertical fluxes to surface fluxes using empirical laws. For further details, see Section 4.3.

In contrast to DNS and LES techniques, statistical and empirical turbulence modeling does not help to understand turbulence itself (Lesieur, 1997). Instead, these techniques are designed for simulating the impact of turbulence and small-scale mixing on the larger scales with reasonable computational effort. A graphical overview over different types of vertical mixing parameterizations is given in Fig. 11.

A special Section 4.4 is dedicated to the modeling of the complex wave–current interaction in the bottom boundary layer. Section 4.5 discusses comparative studies of two-dimensional and three-dimensional model applications to coastal and shelf seas. The problem of horizontal mixing parameterizations for coastal ocean modeling will also be briefly discussed in Section 4.2. Although the meso-scale eddies which are responsible for this type of turbulent mixing should be resolved by the numerical grids, there are some arguments that lateral mixing should still be parameterized, for example, to represent the inverse energy cascade characteristic for

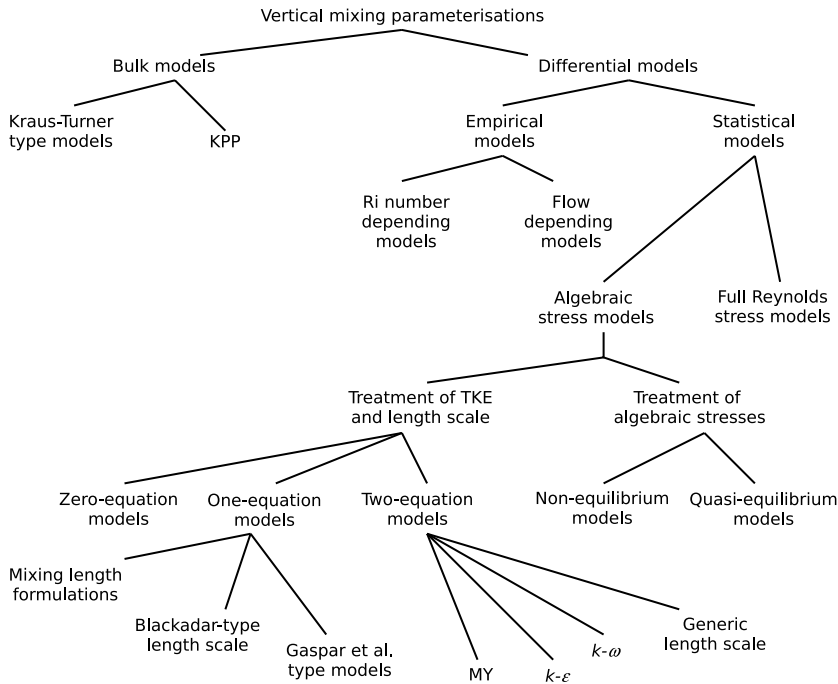


Fig. 11. Diagram showing different types and classes of vertical mixing parameterizations. MY: Mellor–Yamada-type models, KPP: K-profile-parameterisation, TKE: Turbulent kinetic energy. This is an extension of a similar diagram designed by Haidvogel and Beckmann (1999).

geostrophic turbulence. In coastal modeling, it is common practice to use a lateral smoothing operator to damp numerical noise.

#### 4.1. Large eddy simulation

Large-eddy simulation (LES) models move beyond approximate techniques by simulating the largest scales of turbulence using a filtered version of the Navier–Stokes equation. Filtering results in an equation form similar to Reynolds averaging (see Section 4.2), but with a more complicated term representing scales of motion smaller than the grid resolution, commonly referred to as subgrid turbulence.

A typical LES equation of motion for the ocean is

$$\frac{\partial u_i}{\partial t} = -u_j \frac{\partial u_i}{\partial x_j} - \frac{\partial}{\partial x_j} \langle u_i'' u_j'' \rangle - \frac{\partial \tilde{p}}{\partial x_i} + \epsilon_{ijk} (u_j + u_j^s) f_k - \delta_{i3} g \frac{\rho'}{\rho_0} + \epsilon_{ijk} u_j^s \xi_k, \quad (20)$$

where

$$\tilde{p} = \frac{p}{\rho_0} + \frac{2}{3} q^2 + \frac{1}{2} [|u_i + u_i^s|^2 - |u_i|^2], \quad \xi_k = \epsilon_{kij} \left( \frac{\partial u_j}{\partial x_i} \right), \quad (21)$$

and  $\langle u_i'' u_j'' \rangle$  represents the subgrid mixing parameterization. Here,  $u_i$  are the filtered velocity components,  $x_i$  are the Cartesian distances,  $t$  is time,  $g$  is the acceleration due to gravity,  $u_j^s$  are the Stokes drift components representing surface waves,  $f_k$  is the Coriolis term  $\rho'$  is the perturbation density,  $\rho_0$  is the mean density,  $p$  is the pressure,  $q$  is the subgrid scale TKE, and  $\epsilon_{ijk}$  represents the anti-symmetric tensor. Major differences between the LES momentum equation and the standard Navier–Stokes equations include the Stokes drift term, which represents the effect of surface wave orbital motions on the mean currents, and forces LC, and the subgrid term, which accounts for unresolved turbulence. A wide range of subgrid scale parameterizations exist that give excellent results for neutral and convectively forced turbulence. Stable conditions are more problematic because the vertical scales of turbulence are reduced and may fall below the grid resolution. In recent years,

methods that use the original filtered velocity stress equation along with dynamic coefficients have led to improvements for weakly stable turbulence (see Vremen et al., 1997). Nevertheless, very stable conditions that cause intermittent turbulence remain an unsolved problem. Extensive reviews of LES models are given in Galperin and Orzag (1993) and Ferziger and Perić (2001).

Used properly, LES models provide a powerful tool for examining the formation of turbulence and other small-scale processes such as LC (for example, see Skillingstad and Denbo, 1995 or McWilliams et al., 1997). However, LES incurs high computational costs because of grid-size requirements and time-step constraints for small scale disturbances. For example, a typical ocean surface boundary layer thickness is about 30 m. Resolving the largest eddy scales, roughly the depth of the boundary layer, requires about 10 grid points, or a grid resolution of 3 m in both the vertical and horizontal directions. By contrast, even the highest resolution regional coastal models tend to be limited to horizontal grid spacing of 0.5–1 km. Although computer power has been steadily increasing, LES is still only useful for domain scales less than about 2 km.

#### 4.1.1. Coastal applications

Applications of LES have largely concentrated on surface boundary layer processes with open ocean conditions. These studies have typically used open, radiative bottom boundary conditions to account for the deep ocean. Most of the turbulent energy in these simulations comes from surface heat flux, winds and wave forcing. Heat flux generates turbulent motions via convection, whereas wind and waves generate turbulence through shear production and LC. Use of LES in coastal regions is more limited, even though the processes active in the surface boundary layer in the open ocean are also important for coastal waters.

In addition to surface-layer mixing, the coastal ocean also contains a bottom boundary layer that is forced by mean and tidal currents. For a flat, featureless coastal bottom, the lower boundary layer is controlled mostly by shear generated turbulence, much like a neutral ABL. Introduction of obstacles to the flow can lead to a more complicated flow structure, with internal waves and hydraulic jumps (see, for example, Farmer and Armi, 1999; Nash and Moum, 2001). There may also be interaction between the surface boundary layer and the bottom boundary layer.

Almost all LES model applications use periodic lateral boundaries, to avoid the problem of definition of turbulence fields outside the model domain. In the coastal environment, the periodicity assumption is probably reasonable for small regions in which the forcing conditions are relatively uniform, and the coastal bottom slope is small. With the introduction of topography in the form of isolated obstacles, use of periodic boundaries is equivalent to assuming a series of obstacles.

Two scenarios are presented here, with and without topography and stratification, as examples of how LES might be used in coastal regions. The first case represents a uniformly mixed, shallow coastal water column with both bottom roughness and surface forcing. The model is forced with a wind stress of  $0.1 \text{ N m}^{-2}$ , and a surface wave field with 30 m wavelength and 1 m significant wave height (for estimating the Stokes drift  $u^S$ ). A grid domain of 448 points in the horizontal directions and 40 points in the vertical direction is applied with a resolution of 0.5 m.

Cross-section plots (Fig. 12) for this case after 2 h show how coherent structures or LC, produced by the surface wave–current interaction, dominate the flow. Near-surface horizontal scales of LC are typically small relative to the depth of the fluid, but increase rapidly with depth. In the middle of the water column, LC scales are about 25–50 m, or 2–3 times the layer depth. Overall the flow is very turbulent with rapid exchange of water between the surface and bottom.

The introduction of topography and stratification will obviously complicate the dynamics (see, e.g. the observations by Dewey et al., 2005). In the second example, we examine how a simple two-dimensional ridge affects the growth of turbulence in the bottom boundary layer. In this case, the motion is forced by a background current increased from a state of rest over a one-hour period.

The response of stratified flow over an obstacle is determined by a number of non-dimensional parameters that quantify the behavior of internal waves generated by the flow (Baines, 1995). Most important of these parameters is probably the internal wave mode number,

$$K = \frac{HN}{\pi U}, \quad (22)$$

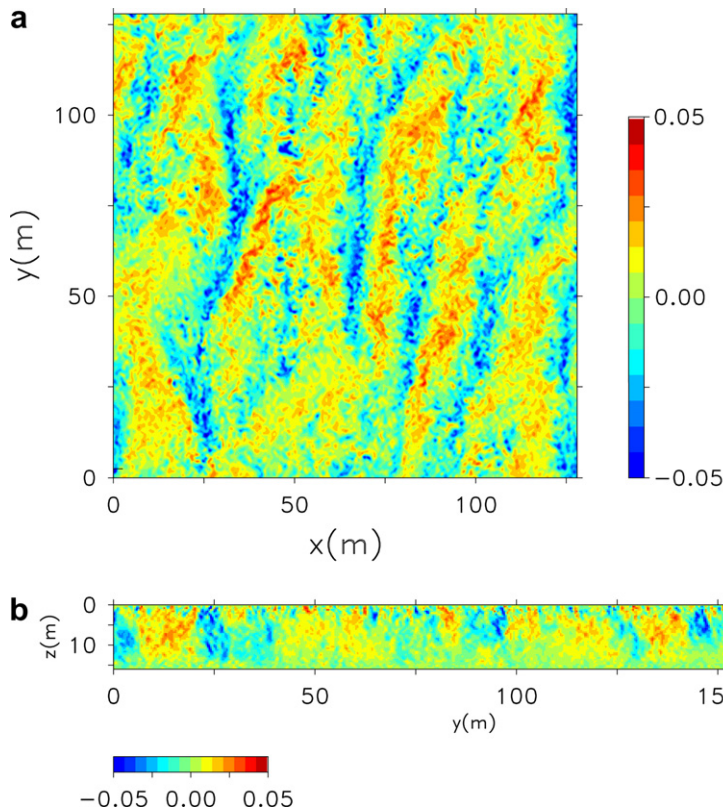


Fig. 12. (a) Horizontal cross-section from 5 m depth and (b) vertical cross-section from  $x = 10$  of vertical velocity ( $\text{m s}^{-1}$ ) after 2 h.

where  $H$  is the water depth,  $N$  is the buoyancy frequency, and  $U$  is the velocity across the obstacle. For a given velocity and stratification, the value of  $K$  tells us what the dominant wave mode will be. For example, when  $K = 1.2$ , we would expect a single vertical wave component, whereas if  $K = 2.2$ , then two wave components would be forced above the obstacle.

Also important for determining the wave behavior is the scaled obstacle height,  $Nh/U$ , which measures the importance of nonlinear effects. For flow with  $Nh/U > 0.85$ , nonlinear effects can cause wave overturning and turbulence production. Scaled obstacle width,  $Na/U$ , is also critical for determining if the flow will generate a train of lee waves ( $Na/U \sim 1\text{--}10$ ) or a more hydraulic or hydrostatic flow behavior ( $Na/U \sim 10\text{--}100$ ) with jump conditions.

Simulations of flow over obstacles in an LES model have only recently been undertaken (see, for example, Calhoun et al., 2001). Here, the LES model used in the flat bottom examples is modified by including topography through a finite volume approach (Adcroft et al., 1997). Using this technique, bottom obstacles are produced by blocking out flow for all grid points below the bottom. For grid points intersected by the bottom terrain, a finite volume is used with mass balance adjusted to account for the smaller fluid volume passing through the grid cell. Advective terms in the equations are also modified by adjusting the flux along the sides of the grid cell (for details see Skyllingstad and Wijesekera, 2004).

Two simulations with differing  $K$  demonstrate how the flow responds to bottom obstacles (Fig. 13). In the first case, a flow velocity of  $0.32 \text{ m s}^{-1}$  and  $K$  value of 1.12 generates a lee wave response with large modulations in the pycnocline layer. When the current is reduced to  $0.16 \text{ m s}^{-1}$ ,  $K$  increases to 2.24 and a mode two internal wave system is produced. The bottom boundary layer displays more structure than a flat bottom case, with large variations in bottom boundary layer depth and turbulence intensity. Furthermore, the mode two structure has altered the pycnocline significantly, generating regions of reduced stratification above the obstacle.

In summary, large-eddy simulation provides a valuable tool for examining basic flow processes that could have a large impact on turbulence generation and the coastal circulation. Most notable is the effect of bottom

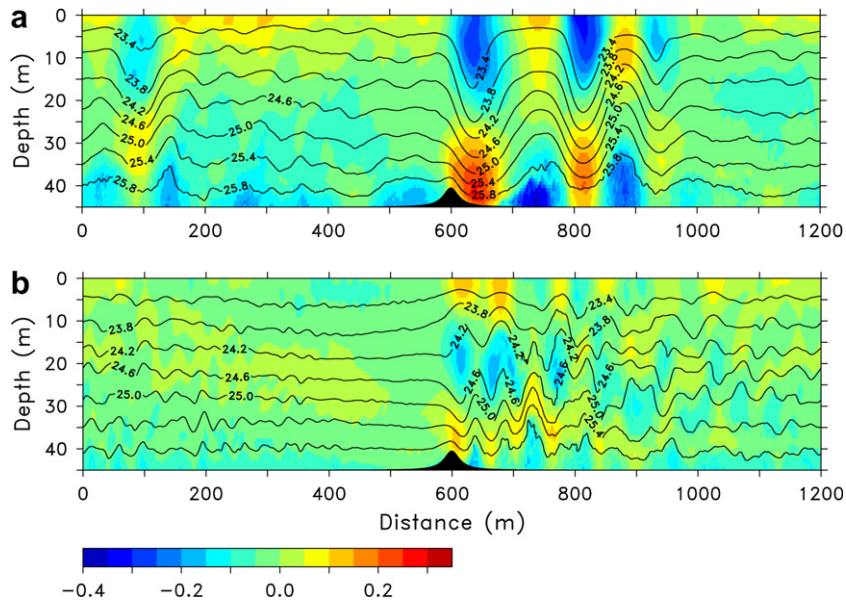


Fig. 13. Vertical cross-sections of horizontal velocity with mean flow subtracted (shaded) and potential density (contours) for background flow of (a)  $0.32 \text{ m s}^{-1}$  and (b)  $0.16 \text{ m s}^{-1}$  after 3 h.

topography, which can greatly enhance regions of mixing both on the ocean bottom and in the pycnocline interior depending on the internal mode characteristics of the flow and the scale of the obstacle. Simulations suggest that smaller obstacles have less impact because they are engulfed by the bottom boundary layer and tend to generate dispersive non-hydrostatic waves. Larger obstacles generate a less dispersive internal wave field, which can become unstable and generate regions of turbulence away from the bottom boundary layer.

Lateral boundaries will require further attention for coastal applications. Typical open boundary models set the inflow conditions to be uniform without prescribing perturbations, except for the outward propagation of internal waves. With a turbulent boundary layer, prescribing inflow conditions is very difficult because the flow has to have a balanced set of conditions describing the inflow of a range of eddy scales. One way to get around this problem is to define a subdomain that is periodic, which then feeds in a balanced turbulent field through the downstream boundary (Mayor et al., 2002). In its present form, this method requires that internal wave energy be absorbed at the model top, which is unrealistic for many ocean situations. (The original Mayor et al. (2002) application was for atmospheric convection where internal waves are of secondary importance.)

## 4.2. Statistical turbulence closure models

### 4.2.1. Basic principles

Almost all macroscopic flow processes can be properly described by the Navier–Stokes equations which are a statement of the conservation law for momentum in a viscous fluid on the rotating earth. These equations are usually simplified by neglecting the direct effect of density differences on momentum (Boussinesq approximation), and assuming incompressibility of the flow (e.g. Sander, 1998). The Navier–Stokes equations, together with conservation laws for mass (the continuity equation), heat and salt, form a closed system of viscous equations for oceanic dynamics. The turbulent motions in the ocean are fully described by this system of equations, which requires statistical treatment to extract the mean properties of the large-scale flow.

State variables (such as velocity, pressure and salinity) are decomposed into a fluctuating part and a mean part. This is the so-called Reynolds decomposition, which needs to fulfil various laws to allow a meaningful statistical treatment of the equations. Most obviously, the averaging procedure must not affect averaged quantities. The only appropriate procedure is the ensemble average, which is theoretically obtained by averaging over an infinite number of macroscopically identical realizations of the same experiment (see Lesieur, 1997).



The Reynolds-averaged Navier–Stokes (RANS) equations are derived by formal substitution of the Reynolds decomposition into the viscous equations, and ensemble-averaging the result. The new set of equations for the averaged quantities is formally equivalent to the Navier–Stokes equations, but contains now several new, unknown, second moments such as the Reynolds stress  $\overline{u'_i u'_j}$ , the turbulent heat flux  $\overline{u'_i T'}$ , and the turbulent salt flux  $\overline{u'_i s'}$  (where subscripts denote spatial directions). As in earlier sections, primes denote fluctuating and overbars averaged quantities. Further transport equations may be derived for these second moments, but third moments then appear in the equations. Repetition of this procedure, deriving transport equations for ever higher moments, leads to the Friedmann–Keller series (Keller and Friedmann, 1924). The series must be truncated at a certain level and the unknown terms closed by means of parameterizations. Second-moment closures retain the second-moment equations and find empirical expressions for the second-order pressure–strain correlators such as  $\overline{u'_i \partial_j p'}$  and the third moments such as  $\overline{u'_i u'_j u'_k}$ . For the pressure–strain correlators a plethora of closures may be found in the literature (see e.g. Mellor and Yamada, 1974; Launder et al., 1975; Kantha and Clayson, 1994; Canuto et al., 2001, 2002; Cheng et al., 2002; Umlauf and Burchard, 2005).

To reduce the number of transport equations, a drastic step is usually taken at this stage: local equilibrium is assumed for the second moments, which are then described by a linear system of equations. This also eliminates the third moments. One price for this significant simplification is neglect of any non-local processes such as counter-gradient fluxes which may occur in convective boundary layers. Furthermore, the effect of the earth's rotation on turbulent motions is neglected, leading to some inaccuracies for deep oceanic convection.

The final step towards a practical turbulence closure model for geophysical flows is the application of the boundary layer assumption which neglects all horizontal mixing terms (see e.g. Burchard, 2002). The result of these closure assumptions is the well-known eddy viscosity principle,

$$\overline{u'w'} = -K_M \partial_z \bar{u}, \quad \overline{v'w'} = -K_M \partial_z \bar{v}, \quad (23)$$

$$\overline{w'T'} = -K_H \partial_z \bar{T}, \quad \overline{w's'} = -K_H \partial_z \bar{s}, \quad (24)$$

with the eddy diffusivity and  $K_M$  the eddy diffusivity  $K_H$  which are calculated as follows:

$$K_M = S_M \left( \frac{k^2}{\varepsilon^2} S^2, \frac{k^2}{\varepsilon^2} N^2 \right) \frac{k^2}{\varepsilon}, \quad K_H = S_H \left( \frac{k^2}{\varepsilon^2} S^2, \frac{k^2}{\varepsilon^2} N^2 \right) \frac{k^2}{\varepsilon}, \quad (25)$$

where  $k$  is the TKE (not to be confused with the wave number  $k$  in Eq. (8)),  $\varepsilon$  is the turbulent dissipation rate and  $S$  and  $N$  are the shear and buoyancy frequency, respectively. The non-dimensional stability functions  $S_M$  and  $S_H$  are complex nonlinear functions of shear- and buoyancy-related non-dimensional parameters. Here, the eddy diffusivities for heat and salt are set to the same values. For a model with different eddy diffusivities, see Canuto et al. (2002). For a comparative study of various sets of stability functions, see Burchard and Bolding (2001) and Umlauf and Burchard (2005). Often, these stability functions are further simplified by a local equilibrium assumption, which allows them to be described as functions of the gradient Richardson number only (Galperin et al., 1988; Kantha and Clayson, 1994).

#### 4.2.2. One- and two-equation models

It should be clearly noted here that so far nothing has been said about the calculation of the TKE,  $k$  and its dissipation rate,  $\varepsilon$ . It is an often repeated misunderstanding that particular calculation methods for  $k$  and  $\varepsilon$  are directly related to particular second-moment closure models. As will be discussed below, it is most important that calculation methods for the stability functions and  $\varepsilon$  have to be consistent. The turbulent quantities  $k$  and  $\varepsilon$  are related through a turbulent length scale  $l$  by the well-known Taylor-scaling,

$$\varepsilon = c_d \frac{k^{3/2}}{l}, \quad (26)$$

where  $c_d$  is an empirical parameter. Given an equation for  $k$ , it is in principle possible to use an equation for any non-linear combination of  $l$ ,  $k$  and  $\varepsilon$  to calculate  $\varepsilon$ . Both the TKE equation and the length-scale related equation may be either full transport equations or algebraic relations. The latter, while simpler, generally lead to lower accuracy and numerical stability.

Transport equations for  $k$  and  $\varepsilon$  can be derived by Reynolds averaging (e.g. Wilcox, 1998). For the TKE equation, closure assumptions have to be made only for the turbulent transport terms,

$$\partial_t k + \partial_z F(k) = K_M S^2 - K_H N^2 - \varepsilon, \quad (27)$$

where the advective transports have been neglected. Vertical turbulent fluxes are denoted by  $F$ . The terms on the right-hand side denote shear production, buoyancy production and dissipation rate, respectively. The closure for production and dissipation terms of the dissipation rate  $\varepsilon$  is not straight-forward. Usually, these terms are based on a normalization of the right-hand side of the TKE equation, as in Rodi (1980). Other length-scale related equations follow this principle. There are presently two models with full transport equations of this form used widely in oceanography: the Mellor and Yamada (1982) model has an equation for  $kl$ ,

$$\partial_t(kl) + \partial_z F(kl) = \frac{l}{2} \left[ e_1 K_M S^2 - e_3 K_H N^2 - \left( 1 + e_3 \left( \frac{l}{l_z} \right)^2 \right) \varepsilon \right], \quad (28)$$

while the  $k$ – $\varepsilon$  model calculates  $\varepsilon$  directly,

$$\partial_t \varepsilon + \partial_z F(\varepsilon) = c_{\varepsilon 1} \frac{\varepsilon}{k} (K_M S^2 - c_{\varepsilon 3} K_H N^2) - c_{\varepsilon 2} \frac{\varepsilon^2}{k}. \quad (29)$$

In Eqs. (28) and (29),  $F$  is the turbulent flux of  $kl$  and  $\varepsilon$ , respectively, and  $e_1$ ,  $e_2$ ,  $e_3$ ,  $c_{\varepsilon 1}$ ,  $c_{\varepsilon 2}$  and  $c_{\varepsilon 3}$  are empirical constants.

Another length-scale related quantity is the turbulence frequency  $\omega = \varepsilon/k$  used in the  $k$ – $\omega$  model (Wilcox, 1988), which has recently been extended for geophysical applications by Umlauf et al. (2003). There is also a new generic two-equation model by Umlauf and Burchard (2003), which uses a transport equation for the quantity  $k^m l^n$ , where  $m$  and  $n$  are non-dimensional exponents.

All empirical parameters occurring in these models must be determined from standard flow situations such as the logarithmic law of the wall for equilibrium turbulence, free decaying turbulence and homogeneous shear flows. Here, the generic two-equation model has the advantage that it contains two more empirical parameters,  $m$  and  $n$ , which allow more flexibility. The buoyancy-related terms in the length-scale equations can be determined from the steady-state Richardson number (which takes a value of about 0.25; see Shih et al., 2000) at which turbulence in homogeneous shear layers should be in equilibrium. Burchard and Baumert (1995) and Baumert and Peters (2001) have shown this for the  $k$ – $\varepsilon$  model and Burchard (2001a) for the  $kl$  model. In both cases, the buoyancy-related parameter depends on the choice for the stability function (e.g. Burchard and Bolding, 2001).

In the so-called  $k$  models, the length-scale related transport equation is replaced by an algebraic relationship for the turbulent length scale ( $l$ ) using Eq. (26) (e.g. Gaspar et al., 1990; Blanke and Delecluse, 1993; Meier, 2001). The physical complexity of the turbulence approach is not necessarily reduced by such an algebraic relationship. If the left-hand side of Eq. (29), representing a memory of dissipation in time and the (controversial) vertical turbulent transport of dissipation, are neglected, the  $k$ – $\varepsilon$  model in its standard form is physically equivalent to a  $k$  model with appropriately chosen length scales (Meier and Kauker, 2002).

#### 4.2.3. Unresolved processes

As mentioned above, the boundary-layer assumption implies neglect of horizontal mixing processes. This is only realistic when the horizontal eddy activity is properly resolved by the numerical model. For large-scale models, this is not the case, and a number of fairly simple subgridscale parameterizations for horizontal mixing have been suggested (e.g. the overview by Haidvogel and Beckmann, 1999).

In the case of coastal ocean models, the meso-scale activity is usually resolved. The size of meso-scale eddies may be approximated by the local internal Rossby radius. Fennel et al. (1991) observed Rossby radii between 1.3 km and 7 km for the Baltic Sea. This means that typical shelf-sea models with resolutions of 3 km are only eddy permitting whereas local coastal models with typically 1 km or less horizontal resolution should be eddy resolving. However, increasing the resolution does not make the eddy closure problem go away but simply

shifts it towards smaller scales, and may even render it more complex as only some effects of geostrophic eddies should be parameterized, to prevent “double counting”.

Eddy closures are still required in regional and coastal models, not only to represent the unresolved physical effects of meso-scale eddies, but also to ensure the numerical stability of the model by preventing enstrophy to build-up at the grid-scale (Roberts and Marshall, 1998). The current practice in coastal modeling is to employ a Laplacian parameterization based on a high-order scale-selective Laplacian operator and a space/flow-dependent eddy coefficient (Smagorinsky, 1963; Leith, 1968; Semtner and Mintz, 1977).

Several processes are not reproduced by the statistical turbulence models and vertical closure assumptions described in this section. Counter-gradient fluxes are excluded because of the neglect of the transport term in the heat flux equation (e.g. the discussion by Zilitinkevich et al., 1999). It has been shown by Canuto et al. (1994) that statistical models closed on the level of the third moments can reproduce convective turbulence. Effects of breaking surface waves (see Section 3.2) may be parameterized by surface fluxes of TKE, contrasting with a wall layer where a zero flux is prescribed (see Craig and Banner, 1994). The Mellor and Yamada (1982) model and the  $k$ – $\varepsilon$  model do not correctly reproduce the vertical structure of the flow in the wave-enhanced layer below the free surface. Thus, some effort has been made to modify these models, for example, by Burchard (2001b) for the  $k$ – $\varepsilon$  model. By contrast with this, the  $k$ – $\omega$  model behaves fairly well under breaking waves and the generic two-equation model can be directly fitted to observed conditions (see Umlauf and Burchard, 2003). LC (see Section 4.1) is not reproduced by these statistical closure models, because of the hydrostatic assumption and the neglect of surface wave effects. There are only a few attempts to empirically include this process in such models, for example, by McWilliams et al. (1997), D'Alessio et al. (1998), Axell (2002), Kantha and Clayson (2004). Finally, the most critical process not reproduced by statistical turbulence models is the activity of internal waves and their interaction with turbulence (see Section 2.3). After the Reynolds decomposition, internal waves are in principle included in the RANS equations, but later partially excluded by the hydrostatic assumption and the model resolution. Thus far, the representation of internal wave mixing in ocean models has been relatively crude (e.g. Mellor, 1989; Large et al., 1994; Meier, 2001; Axell, 2002; St. Laurent and Garrett, 2002).

#### 4.2.4. Example application

Many of the above-mentioned turbulence closure models have been implemented into the Public Domain water column model General Ocean Turbulence Model (GOTM) which is published via the internet at <http://www.gotm.net> (Burchard et al., 1999). GOTM has been applied for simulating various oceanic and limnic scenarios, one of them being the Liverpool Bay 1999 campaign reported and analyzed by Rippeth et al. (2001). The major process observed at this site was the strain-induced periodic stratification (SIPS), and the modeling challenge was to see whether two-equation turbulence models are able to reproduce the convective mixing involved. For this purpose, Simpson et al. (2002) simulated the Liverpool Bay scenario with a  $k$ – $\varepsilon$  model coupled to the second moment closure by Canuto et al. (2002).

For the simulation, a barotropic tide, surface fluxes of heat and momentum, horizontal gradients of temperature and salinity (assumed to be constant in time and space) and initial conditions for temperature, salinity and momentum had to be prescribed from observations. The simulations showed a slight phase shift for temperature, salinity and momentum compared with the observations, probably as a result of simplistic assumptions about the horizontal density gradients. Since the focus of the simulations was to evaluate the ability of the model to reproduce turbulence dynamics, the simulated temperature and salinity were relaxed to observations (Simpson et al., 2002). Fig. 14 shows the resulting temperature, salinity and momentum compared with observations. The good agreement between observed and simulated temperature and salinity is surely due to the relaxation, which can be regarded as correcting the advection of these quantities. The simulated velocity components, also agree quite well with the observations.

Fig. 15 shows observed and simulated dissipation rate. Quantitatively and qualitatively, the agreement is quite good. In particular, the tidal asymmetry with strongly enhanced turbulence during flood and suppressed turbulence during ebb is clearly reproduced. This result, which has been obtained without any specific tuning, gives some credibility to statistical turbulence models and their applicability in complex coastal and estuarine flow situations.

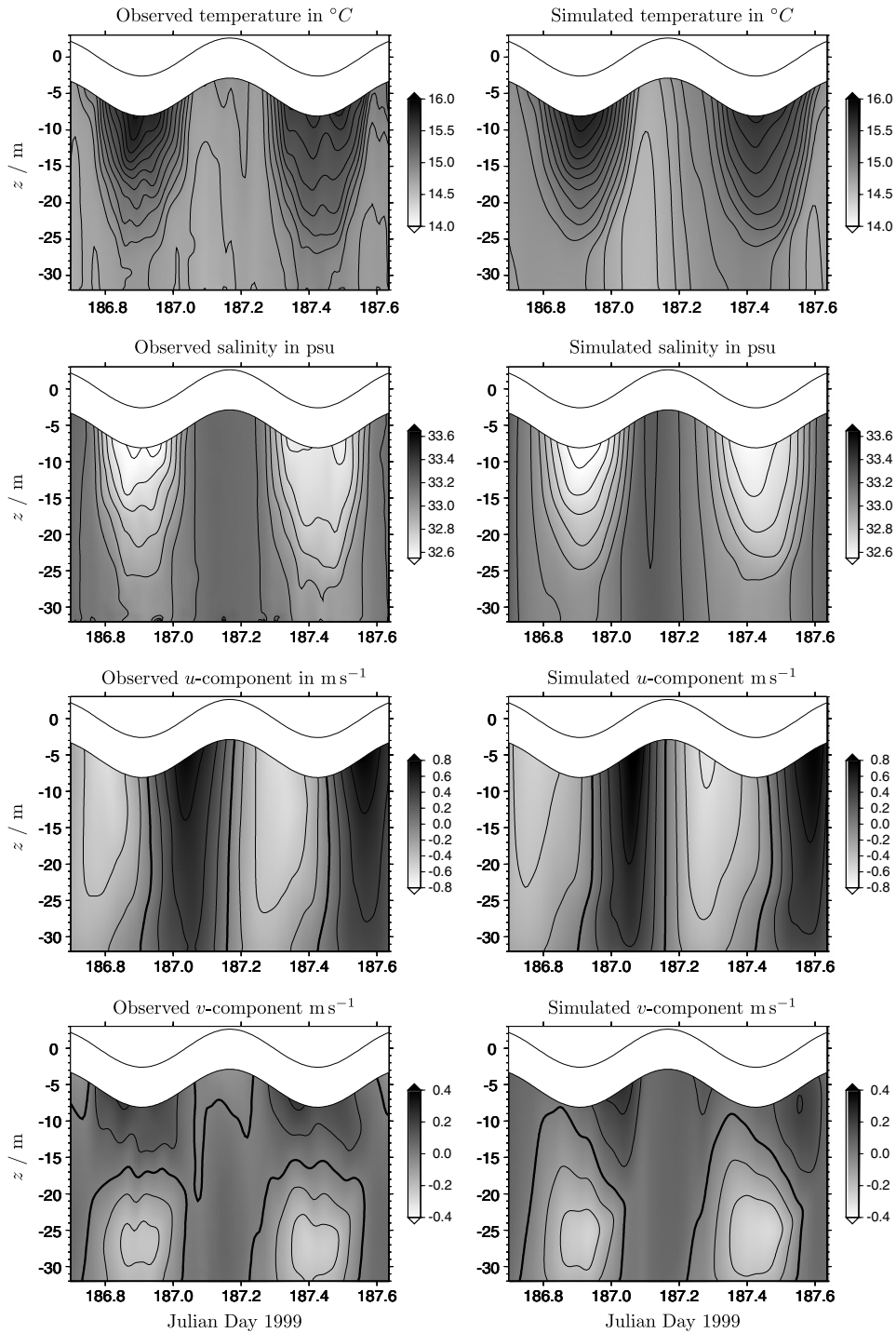


Fig. 14. Strain-induced Periodic Stratification (SIPS) in Liverpool Bay. Temperature,  $\bar{T}$  salinity  $\bar{s}$  and velocity components  $\bar{u}$  and  $\bar{v}$  from observations by Rippeth et al. (2001) (left panels) and simulations with nudging to observed  $\bar{T}$  and  $\bar{s}$  (right panels) for two tidal cycles. Note the complete mixing at high water (after flood) and the significant stratification at low water (after ebb). The  $u$ -component of the velocity is onshore, the  $v$ -velocity is offshore. It can be clearly seen in the  $u$ -velocity profiles how the ebb profiles are significantly sheared throughout the water column (due to suppression of turbulence by stable stratification), and how the opposite (small shear in the upper parts of the water column) is seen during flood. This figure has been taken from Burchard (2002).

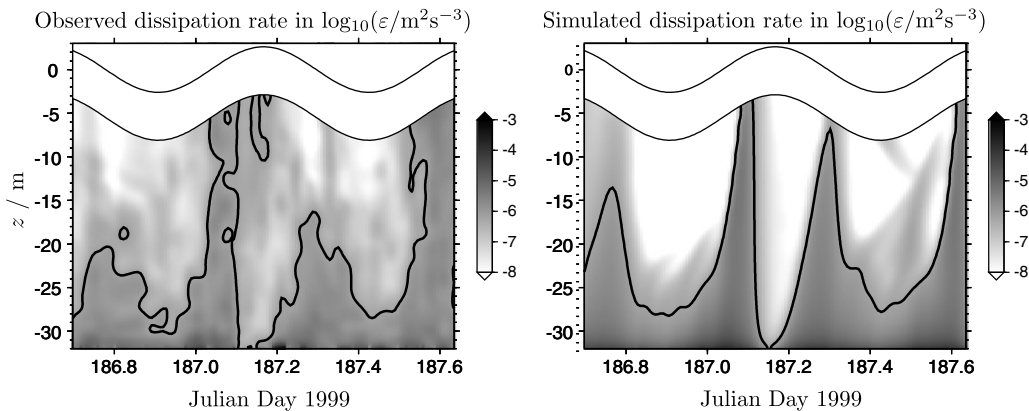


Fig. 15. Strain-induced Periodical Stratification (SIPS) in Liverpool Bay: turbulent dissipation rate in  $\text{W kg}^{-1}$  from observations by Rippeth et al. (2001) (left panel) and simulations with nudging to observed temperature and salinity (right panel) for two tidal cycles. For better interpretation, the  $-6.2$  isoline is drawn as a bold line. It can be clearly seen that the high turbulence region (expressed as high dissipation rates) reaches up to the surface during flood and only to about midwater during ebb. This figure has been taken from Burchard (2002).

#### 4.3. Empirical turbulence models

Like the statistical models described in Section 4.2, empirical models for coastal turbulence are based on the RANS equations simplified using the boundary layer approximation. However, the modeling of turbulent fluxes differs dramatically from the closure approach. Instead of approximating the fluxes systematically in terms of a Friedman–Keller series, the turbulent formulae are based entirely on empirical knowledge of fluxes in geophysical boundary layers.

The main weakness of the empirical approach is that it is only as good as the observations used to model the fluxes. Even if the observations were perfectly accurate, the parameterizations are necessarily based on only a limited number of particular cases. Thus, the empirical approach demands substantial faith in the generality of geophysical boundary layer physics. (The same could be said of statistical approaches, but the problem is more severe in empirical modeling as the latter, by definition, relies almost entirely on observational evidence to justify the flux parameterizations.)

This weakness is offset by several advantages. First, the empirical approach facilitates the inclusion of physical processes that are not captured naturally by a truncated Friedman–Keller series (e.g. the non-local fluxes described below). A more practical advantage is that empirical turbulence parameterizations do not require the incorporation of additional prognostic variables such as TKE. This results in a substantial saving in both computer memory and processing time.

The earliest empirical models treated the boundary layer as a slab (e.g. Kraus and Turner, 1967; Denman, 1973), and that assumption is retained in some current models (e.g. Price et al., 1986). Here, we focus on a more recent model that, among other advances, allows boundary layers to assume a more realistic vertical structure. This is the non-local  $K$ -profile parameterization (KPP), introduced by Large et al. (1994). The KPP is also distinguished from other empirical models by the inclusion of non-local fluxes. These fluxes are driven by eddies on scales comparable to or larger than the vertical scale over which the background shear and stratification change. In the nocturnal surface boundary layer, for example, net surface cooling generates a mean density gradient that is unstable near the surface, nearly zero within the mixed layer, and strongly stable in the entrainment zone at the base of the mixed layer. Convective plumes carry property fluxes that depend on the net buoyancy change across the mixed layer, and therefore have little relationship to the local gradient at any particular depth. Such fluxes can be strong even in the mixed layer interior, where the background gradients are near zero. Parameterization of these fluxes using the standard gradient-diffusion formalism (e.g. Eqs. (23) and (24) of Section 4.2) is clearly inappropriate.



#### 4.3.1. The $K$ -profile parameterization

The KPP was described first and in greatest detail by Large et al. (1994). Since then, a number of studies have led to refinements of the model: Large and Gent (1999), McWilliams and Sullivan (2001), Smyth et al. (2002) and Wijesekera et al. (2003). The reader may find details of the model in any of these publications; here, we will describe only its essential structure.

The KPP treats boundary and interior regions separately. Parameterizations are needed to effect this separation as well as to approximate the fluxes within each region. For simplicity, we describe the parameterization of fluxes in a surface boundary layer and in the interior region beneath it. Application in coastal regions demands the additional consideration of bottom boundary layers, an extension that is straightforward in principle but more complicated in practice (Durski et al., 2002; Wijesekera et al., 2003).

The boundary layer and the underlying thermocline are separated by the surface  $z = -h(t)$ . (The vertical coordinate is positive upwards and zero at the ocean surface). The boundary-layer depth  $h(t)$  is defined as the shallowest depth  $z$  at which the bulk Richardson number,  $R_b(z, t)$  (see Large et al., 1994), exceeds a specified critical value  $R_{ic}$ . The bulk Richardson number is computed in terms of differences in density and horizontal velocity between the given depth and the surface layer (defined most simply as the upper tenth of the boundary layer).

The vertical turbulent flux  $\overline{c'w'}$  of any fluctuating quantity  $c'$  is expressed as the sum of a local and a non-local part, represented as

$$\overline{c'w'} = \overline{c'w'_L} + \overline{c'w'_N}. \quad (30)$$

The local component is given by the usual gradient parameterization,

$$\overline{c'w'_L} = -K_c \frac{\partial C}{\partial z}, \quad (31)$$

where  $C(z, t)$  represents a mean field quantity whose fluctuating part is  $c'$  and  $K_c(z, t)$  is the corresponding turbulent diffusivity. Parameterization of the diffusivity  $K_c$  is described next; non-local fluxes are discussed subsequently.

Below the boundary layer ( $z < -h$ ), fluxes are entirely local and are defined using simple parameterizations of three classes of processes: internal-wave breaking, shear instability and double diffusion. Wave breaking is represented by a constant diffusivity that depends only on the quantity represented by  $c$ , i.e. momentum or a scalar concentration. The diffusivity due to shear instability is the same for all fields and is represented by a simple function of the local gradient Richardson number. Modifications to the original function of Large et al. (1994) have been described by Large and Gent (1999). Double diffusion is parameterized in terms of the ratio,  $R_\rho$ , of thermal to saline components of the density stratification.

Within the surface boundary layer ( $z > -h$ ), the diffusivity  $K_c$  is parameterized as the product of the boundary layer depth, a turbulent velocity scale and a vertical shape function,

$$K_c = h(t) \cdot w_c(z, t) \cdot G_c(z, t) \quad (32)$$

The velocity scale is proportional to the friction velocity  $u^*$  and is modified near the surface in accordance with similarity scaling. In recent studies (McWilliams and Sullivan, 2001; Smyth et al., 2002), the velocity scale has been modified further to account for mixing by Langmuir cells. The shape function is a cubic function of depth with coefficients chosen to ensure continuity of fluxes at both the surface and the base of the boundary layer.

The non-local flux of the property  $c$  is the flux due to eddies comparable in size to the scale on which the background gradient  $\partial C / \partial z$  varies. Non-local fluxes may be written in terms of local variables by making various approximations to the flux budget (e.g. Deardorff, 1972; Therry and Lacarrere, 1983; Holtzag and Moeng, 1991). In low-order models those fluxes must be parameterized in terms of resolved variables. The parameterization is contained within an effective “gradient”,  $-\gamma_c(z, t)$ , that is added to the background gradient of  $c$ ,

$$\overline{c'w'_N} = K_c \gamma_c. \quad (33)$$

The effective gradient is defined such that the product  $K_c \gamma_c$  depends only on surface fluxes and the surface layer thickness. Large et al. (1994) established parameter values for non-local fluxes of heat and salt. Parameterization of non-local momentum fluxes has been undertaken more recently by Smyth et al. (2002).

The choice of turbulence closure scheme will depend on the application. For the wave-affected bottom boundary layer, discussed in Section 4.4, early theories were developed using an empirical scheme, but most recent studies are moving towards high-resolution statistical schemes. Two cases studies illustrating implementations of the KPP are given in Section 4.5.

#### 4.4. Modeling the wave–current bottom boundary layer

In the coastal ocean, the effects of surface waves may be felt right to the seabed. As already discussed in Section 2.2, the turbulent structure of a wave-induced bottom boundary layer differs significantly from the law of the wall. The presence of the wave-induced boundary layer modifies the thicker boundary layer due to the lower-frequency currents, and thus affects the currents right through the water column.

There are two common modeling approaches to account for the boundary-layer interaction between high-frequency waves and the mean flow. The first, exemplified by Grant and Madsen (1986), considers the wave motion as subgrid scale and parameterizes its impact on the mean current field. The second seeks to resolve the waves' space and time scales in the numerical model.

##### 4.4.1. The Grant and Madsen approach

Numerous analytical models have been proposed for the structure and interaction of co-existing wave and current boundary layers. The most commonly used models incorporate the influence of the wave boundary layer in the turbulence parameters of the mean flow, particularly the eddy viscosity and roughness length. This is generally done using a multi-layered approach, where different characteristic turbulent and length scales are considered appropriate in different boundary regions. In their classic model, Grant and Madsen (1986) used an empirical, two-layer, time-invariant eddy viscosity, with a wave bottom boundary layer and an overlying wall layer. Both layers have logarithmic current profiles, but with different effective roughness lengths. The apparent roughness length for the upper layer is calculated from the requirement of continuous current profiles. This leads to a discontinuity in the eddy viscosity at the top of the wave boundary layer. The interdependency between the wave- and current-related parameters also requires an iterative solution technique.

Some investigators have proposed less complex eddy viscosity based models that do not require iterative solution. Signell et al. (1990) assumed that currents have a negligible effect on the wave boundary layer. You (1994) reported encouraging comparisons to data when neglecting the angle between wave and current directions, and using a 3-layer eddy viscosity profile similar to Madsen and Wikramanayake (1991).

Most recently, Styles and Glenn (2000) extended the Grant–Madsen model to include water-column stratification due to suspended sediment, leading to reduced vertical mixing. The net result of significant stratification is an increase in apparent bottom roughness and reductions in bottom shear stress, currents and sediment concentrations. Calculations for typical inner shelf storm conditions showed that their stratification correction ultimately reduced sediment transport rates by almost two orders of magnitude. The possible range in values of poorly established model parameters also had an order of magnitude impact on these predictions.

The verification of sediment stratification theory is unfortunately problematic. A number of the parameters are difficult to measure and accurate predictions rely heavily upon accurate estimations of physical bed roughness and near-bed reference concentration, neither of which is easily determined in its own right. Physical bed roughness is an uncertainty in all wave–current boundary-layer models. If it is calculated from bed shear stress, rather than held constant, one more level of iteration is necessary in the model solution scheme.

Welsh et al. (2001) outline the computational and physics difficulties that can result when bed roughness, wave–current boundary layer and sediment stratification parameterizations are combined to form a multiply-iterative solution scheme. In a related paper, Welsh et al. (2002) present results from Adriatic Sea simulations using coupled wave, hydrodynamic and sediment transport models. The effects of the bottom boundary layer couplings include 30% reductions in coastal currents and storm surge, and widespread onset of sediment suspension due to wave-dominated forcing in water depths less than 30 m.

#### 4.4.2. High-resolution modeling

Modeling with high vertical resolution has the potential to represent both the wave and turbulence bottom-boundary layers. The advantages of such an approach over analytical boundary layer parameterizations include

- the prediction of boundary layer evolution over a wave cycle and the associated phase lag relationships;
- applicability across the continuum from wave-dominated to current-dominated conditions; and
- no assumptions regarding steady-state conditions or local turbulence equilibrium.

The time steps and near-bed vertical grid resolutions used in the investigations cited above are on the order of 0.1 s and 0.1 mm. These levels of resolution are at this time impracticably small for use in three-dimensional basin scale or even coastal scale simulations.

Early approaches, usually in 1 or 2 dimensions (e.g. Sheng, 1984; Hagatun and Eidsvik, 1986; Davies et al., 1988), tended to confirm the Grant and Madsen (1986) approach. Most recently, Davies and Villaret (2002) added bed ripple roughness calculations and a related vortex-shedding sub-model to the “flat bed” model of Li and Davies (1996). These modifications substantially improved agreement with field data for sediment transport, confirming the crucial importance of accurate bed roughness estimates. Transport predictions were also made using the relatively simple, analytical model of Bijker (1971), with ripple calculations again included. Overall, the TKE model predictions for mean concentration profiles and net longshore sediment transport rates were only slightly better than those of the “engineering” model. The ability of a numerical model to resolve complex, intra-wave cycle phenomena was again demonstrated through the prediction of cross-shore grain-size sorting as a consequence of the greater turbulence intensity in the “wave plus current” half-cycle.

Mellor (2002) carried out one-dimensional simulations of pure wave and wave–current boundary layers, respectively, using TKE and turbulent length scale transport equations. The pure wave boundary layer results agreed well at all phases with laboratory data in terms of current profile, shear stress profile, TKE profile and bottom stress. On this basis, the results from the wave–current simulations were used to develop two alternative parameterizations of wave effects for use in large-scale circulation models. The numerical simulations indicated that the key impact of waves was an increase in turbulence levels. An additional shear production term was therefore derived for inclusion in the TKE transport equation. This approach also requires a minor modification of the normal “law of the wall” boundary condition. For circulation models that do not include a TKE equation, an alternative parameterization for  $z_{0a}$  was developed. It is noted, however, that the use of a  $z_{0a}$  approach will unavoidably result in poor mean current predictions very close to the bed (on the order of a cm).

#### 4.5. Sensitivity of 2D and 3D model results to turbulence parameterizations

In the following subsections, two case studies for coastal and shelf-sea applications are discussed as examples of different turbulence implementations in hydrodynamic models. The first is an application of statistical turbulence closure to the Baltic Sea, and the second a case of coastal upwelling and downwelling using statistical turbulence closures and the KPP-model.

##### 4.5.1. A case study for the Baltic Sea

The Baltic Sea is a large estuary forced by freshwater surplus from river runoff and net precipitation. The water exchange with the world ocean is restricted by the narrows and sills of the Danish Straits. In the long-term, high-salinity water from the Kattegat enters the Baltic proper and low-salinity water leaves the estuary due to the freshwater surplus. A permanent halocline, located deeper than the sills in the entrance area, separates the homogeneous upper layer and the stratified deep water. The direct influence of wind-induced mixing and of convection generated by surface buoyancy fluxes is limited to the upper layer by the strong stratification.

Mixing plays a dominant role in determining both the depth of the seasonal thermocline and the vertical circulation in the Baltic Sea on annual to decadal time scales, see the recent review by Reissmann et al. (in press). The inflowing high-salinity water from the Kattegat is modified by mixing in the shallow entrance area

and by entrainment on its path into the Baltic proper, see the observational studies by Sellschopp et al. (2006), Arneborg et al. (2007) and Umlauf et al. (2007).

**4.5.1.1. The seasonal cycle of the thermocline.** In general, the seasonal thermocline is simulated correctly if mixing in the ocean is calculated utilizing one of the turbulence models described in Sections 4.2 and 4.3 (see also Fig. 16).

In summer, thermocline depths are slightly underestimated. In autumn, the model results show a tendency to overestimate temperature gradients within the thermocline. Especially during the severe winters 1984/1985, 1985/1986 and 1986/1987, the formation of the so-called Baltic winter water with temperatures below the temperature of maximum density is underestimated. The performance of the  $k$  model with appropriately chosen length scales and of the standard  $k$ – $\epsilon$  model (Eqs. (27) and (29)) are almost identical.

**4.5.1.2. Decadal variability of the permanent halocline.** Fig. 17 shows the time evolution of a vertical salinity profile in the eastern Gotland Basin (Gotland Deep) for the period 1902–1998. The hydrography at Gotland Deep is representative of the main basin of the Baltic Sea. Shown are three simulations utilizing either the standard  $k$ – $\epsilon$  model or the  $k$  model presented in Section 4.2. In the last experiment the surface wind speed has been increased by 30% because only daily sea level pressure and wind fields have been considered.

Results of the two turbulence parameterizations, the standard  $k$ – $\epsilon$  model (Fig. 17b) and the  $k$  model (Fig. 17c), are quite different. The standard  $k$ – $\epsilon$  model causes significantly underestimated salinity in the lower

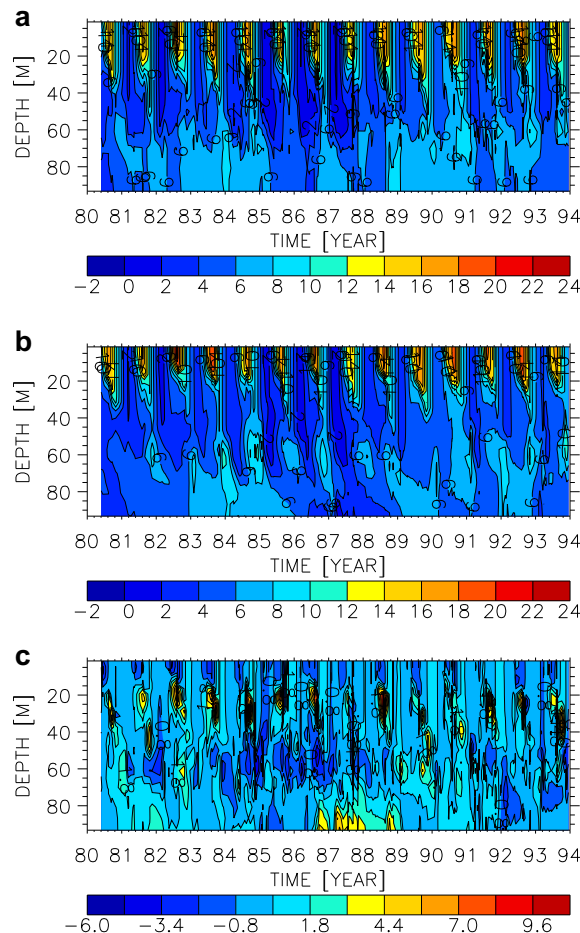


Fig. 16. Isotherm depths (in °C) from May 1980 to December 1993 in the western Baltic Sea (Bornholm Deep): (a) observations, (b)  $k$ – $\epsilon$  model results Meier (2001), and (c) difference between data and model results.

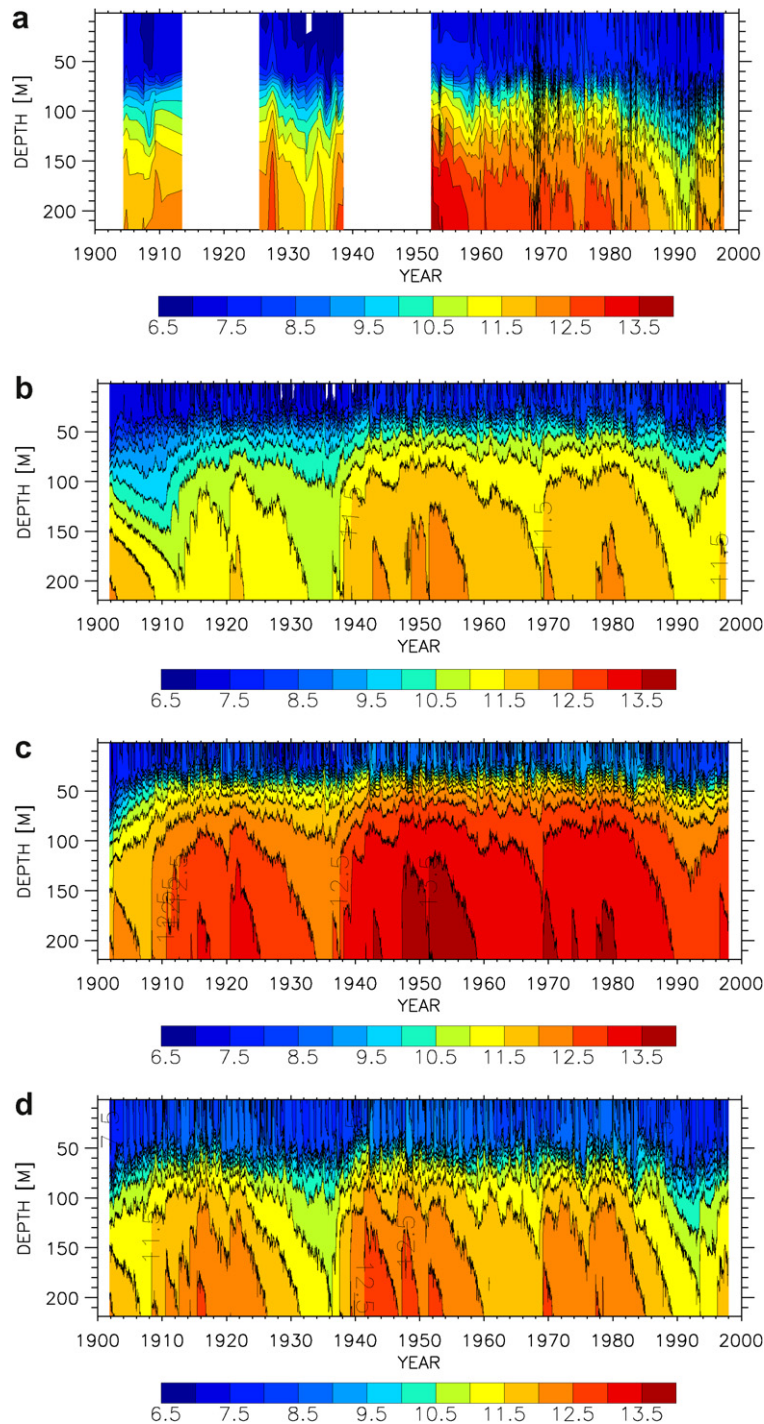


Fig. 17. Isohaline depths (in psu) in the eastern Gotland Basin: (a) observations, (b) standard  $k-\epsilon$  model, (c)  $k$  model, and (d)  $k$  model with 30% increased wind speed. Observations are not available for periods covering the world wars. The simulations using a Baltic Sea model with a horizontal grid distance of 6 nm are described by Meier and Kauker (2002).

layer, while the  $k$  model results in significant overestimates. In both simulations the halocline is much too shallow, causing a model drift towards higher salinities, especially during the first two decades in all simulations. The agreement between the  $k$  model with increased wind speed (Fig. 17d) and observations (Fig. 17a) is sat-



isfactory. Minima of lower layer salinity during the 1920/1930s and during the 1980/1990s and the pronounced maximum during the 1950s are simulated correctly. The depth of the halocline and maximum lower-layer salinity during the 1950s are still underestimated, and the surface-layer salinity is somewhat too high.

The low vertical mixing in the model may follow from exclusion of unresolved processes such as LC and internal waves. The modelled flux of energy from the wind into inertial motions may also be biased. The three-hour resolution may cause an underestimation of the energy flux by 20% (D'Asaro, 1985). As the energy flux to inertial motions is proportional to the fourth power of the wind speed, only the tail of the distribution for high wind speeds is important for mixing. A few winter storms may provide all of the energy necessary to deepen the surface mixed layer as observed.

#### 4.5.2. Coastal upwelling and downwelling

Wijesekera et al. (2003) examined the sensitivity of model-produced time-dependent upwelling and downwelling circulation on the Oregon continental shelf to turbulent closure scheme employed with a two-dimensional approximation (variations across-shelf and in depth) using the Princeton Ocean Model (POM; Blumberg and Mellor, 1987). The level 2.5 Mellor–Yamada closure (Mellor and Yamada, 1982; Galperin et al., 1988; Kantha and Clayson, 1994),  $k$ – $\epsilon$  closure (Rodi, 1987; Burchard et al., 1998), and KPP were used to evaluate eddy fluxes in the circulation model. The model was forced by idealized, constant wind stress with

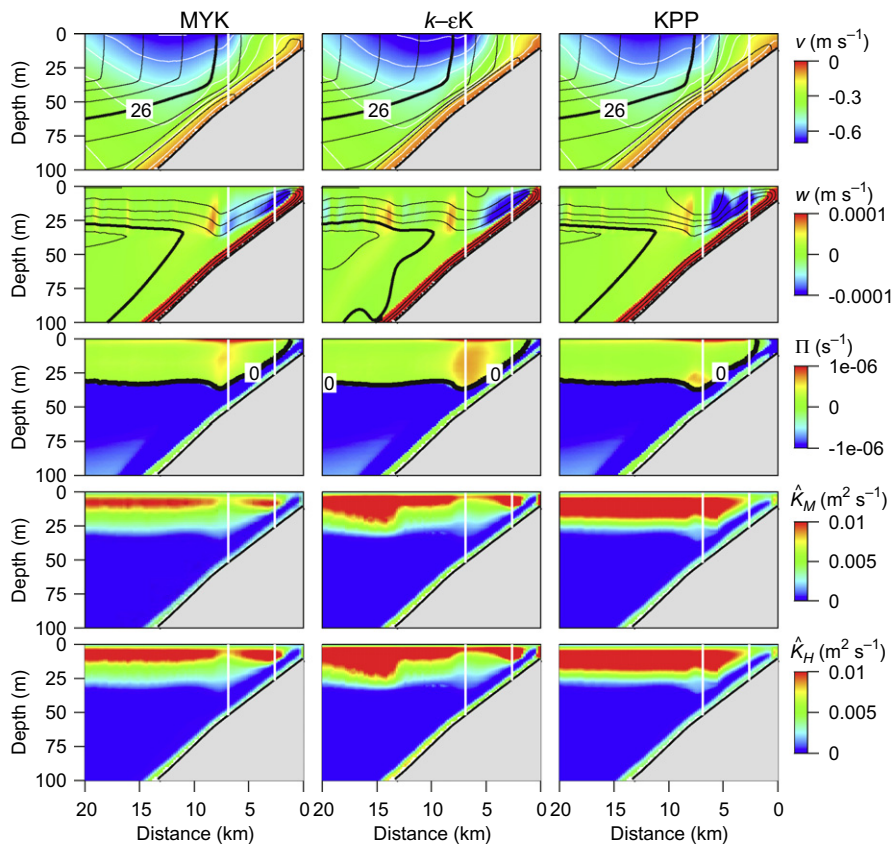


Fig. 18. Model results for the upwelling case, illustrated by fields of alongshore velocity  $v$  (color contours and white lines) together with density  $\sigma_\theta$  (black contour lines), vertical velocity  $w$  (color contours) together with the stream function for the across-stream flow  $\Psi$  (black contour lines), potential vorticity  $\Pi$ , eddy viscosity  $K_M$  and eddy diffusivity  $K_H$  for the Mellor–Yamada ( $k$ – $kl$ ),  $k$ – $\epsilon$  and KPP mixing schemes after day 17. All variables are averaged over an inertial period. The contour intervals are  $\Delta\sigma_\theta = 0.2 \text{ kg m}^{-3}$ ;  $\Delta v = 0.1 \text{ m s}^{-1}$ ,  $\Delta\Psi = 0.1 \text{ m}^2 \text{ s}^{-1}$ , where  $\Psi = 0.4 \text{ m}^2 \text{ s}^{-1}$  is marked with a heavy line. Both  $k$ – $kl$  and  $k$ – $\epsilon$  closures used parameters specified by Kantha and Clayson (1994). (Adapted from Wijesekera et al., 2003.)

no surface buoyancy flux. The topography was chosen to approximate the Oregon continental shelf and slope at latitude 45N. Here, we review results from two cases in which wind forcing was upwelling- and downwelling-favorable.

*Upwelling case:* All three closure schemes produced similar features in the mesoscale circulation including upwelling near the coast and a southward coastal jet (Fig. 18). The across-shelf circulation pattern shown by the stream function  $\Psi$  illustrates offshore Ekman transport in the surface boundary layer, with onshore transport concentrated in the bottom boundary layer. Although the thickness of boundary layers is similar for all three schemes, the structure within these layers depends upon the submodel.

*Downwelling case:* Again, all three models produce a similar mesoscale circulation over the shelf (Fig. 19), now including downwelling front, coastal jet and symmetric instabilities (e.g. Allen and Newberger, 1998) in the bottom boundary layer. The velocity front is sharpest in the  $k$ - $\epsilon$  model, with  $k$ - $kl$  and KPP velocity fronts comparable. Although the velocities are in good agreement for all three schemes, there is a small offshore flow at the surface and onshore flow near the bottom for KPP, which can be seen in  $\Psi$  field of Fig. 19. On the inner-part of the shelf, potential density is vertically uniform and surface boundary layer extends to the bottom. For the KPP scheme this implies that the mixing coefficients are determined for the full depth by the non-dimensional shape function  $G_x$  which results in greater mixing in the interior than in the  $k$ - $kl$  and the  $k$ - $\epsilon$  closure.

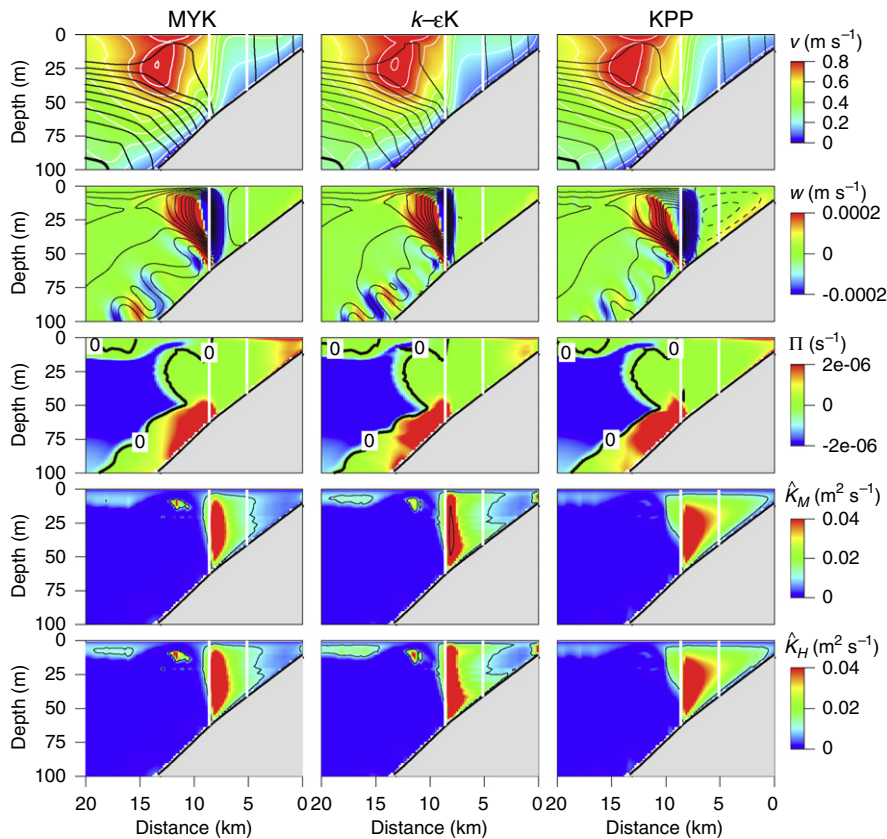


Fig. 19. Model results for the downwelling case, illustrated by fields of alongshore velocity  $v$  (color contours and white lines) together with density  $\sigma_\theta$  (black contour lines), vertical velocity  $w$  (color contours) together with the stream function for the across-stream flow  $\Psi$  (black contour lines), potential vorticity  $\Pi$ , eddy viscosity  $K_M$  and eddy diffusivity  $K_H$  for the Mellor–Yamada ( $k$ - $kl$ ),  $k$ - $\epsilon$  and KPP mixing schemes after at day 17. All variables are averaged over an inertial period. The contour intervals are  $\Delta\sigma_\theta = 0.2 \text{ kg m}^{-3}$ ;  $\Delta v = 0.1 \text{ m s}^{-1}$ ,  $\Delta\Psi = 0.1 \text{ m}^2 \text{ s}^{-1}$ , where  $\Psi = 0 \text{ m}^2 \text{ s}^{-1}$  is marked with a heavy line and  $\Psi > 0$  is dashed. Both  $k$ - $kl$  and  $k$ - $\epsilon$  closures were used parameters specified by Kantha and Clayson (1994). (Adapted from Wijesekera et al. (2003).)

#### 4.5.3. Case study overview

For the Baltic Sea studies, vertical diffusion of TKE seemed to play an important role, so that local schemes, like Richardson-number-dependent friction (Pacanowski and Philander, 1981), are probably not sufficient to parameterize the mixing. It is not possible from this study to decide if a  $k$  model with appropriately chosen length scales or a  $k$ – $\epsilon$  model approach is superior for the coupling with an circulation model. More important for the performance are the details of the turbulence model, such as the value of the parameter  $c_{\epsilon 3}$ , the choice of the stability functions, and how vertical diffusion of dissipation in the ocean interior is included (see also the discussion by Burchard et al., 1998).

The study by Wijesekera et al. (2003) provides one of the few comparisons of the KPP model to statistical turbulence closures for two- (or three-) dimensional models. In this case, there are no significant differences between the results obtained with the different schemes, although details in structure of the boundary layers varied. However, further comparisons over a broader range of initial conditions by Durski et al. (2002) found marked differences between solutions with the Mellor–Yamada level 2.5 closure and the K-profile parameterization in simulations of shallow stratified water. These differences were related to both the capability for TKE to diffuse in the Mellor–Yamada scheme and the arguably more appropriate relationship established between vertical mixing and gradient Richardson number of the KPP.

The two examples discussed in this section show that general recommendations on the use of a certain turbulence closure model within three-dimensional models cannot be given. For fairly simple situations such as the dynamics of coastal upwelling and downwelling, differences between empirical and statistical closure models and between various statistical closures are difficult to identify. For more complex situations such as the balance between stratification and mixing in the Baltic Sea over long time scales and in different regions (narrow straits versus Baltic proper), the results of three-dimensional models are sensitive to the choice of the embedded turbulence model. This sensitivity might be dependent on the circulation model itself (e.g. grid resolution), because the simulated vertical velocity shear is the most important source of TKE below the turbulence-enhanced surface layer (Meier, 2001).

## 5. Conclusions

Recent years have seen considerable advances in our ability to understand and quantify turbulent fluxes in coastal seas. The development of new observational methods and modeling techniques has yielded deeper insight into turbulent phenomena. LC is a case in point. Investigation of Langmuir dynamics has required innovative instrumentation, such as autonomous underwater vehicles (Thorpe et al., 2003b) and surface-tracking profiling instruments (Gemmrich and Farmer, 1999a), together with new modeling tools such as LES (Section 4.1 and the citations therein).

Surface and internal waves complicate the measurement and analysis of coastal ocean turbulent fluxes. For the interaction of breaking surface waves with near-surface turbulence, different observational techniques yield different results and insights. Surface-tracking instruments resolve thinner near-surface layers than free-rising profilers or instruments mounted on poles (Section 3.2). As long as consistent observational data sets are missing, the development of mathematical models for the reproduction of near-surface processes will also be delayed. For example, the model by Craig and Banner (1994) has not yet been significantly improved, although it has raised many questions concerning, for example, whether  $l$  scales as 0.4 or as 0.2 times the distance from the surface (Umlauf and Burchard, 2003), the functional approximation of turbulent quantities such as TKE and its dissipation rate, and the thickness of the unresolved layer. The impact of surface waves in shallow water on the near-bed currents are understood in principle (Section 2.2), but the consequences for mean transports of mass and suspended matter and the interaction with the sediment remain partially unresolved. The Grant and Madsen (1986) model, which is conceptually difficult to couple with turbulence models, has survived for more than two decades until its recent challenge from Mellor (2002).

For the interaction between internal waves and turbulence, the situation is even worse. Internal waves are not resolved in most models, and their parameterization seems to be complicated by their non-local character: they may be generated far away from the location where they break and produce turbulence. Shelf and coastal seas are more complicated because internal waves are not free, but are forced by tidal straining (Section 2.3).

Usually, modellers use simple “internal wave” parameterizations (Sections 4.2 and 4.3), the simplest being a prescription of constant background diffusivity.

In situ, two-dimensional PIV is one of the more innovative new tools for measuring turbulent processes (Section 3.4). It is the only method that resolves turbulent motions in more than one spatial dimension. This eliminates otherwise speculative assumptions on the anisotropy of turbulence. In situ three-dimensional particle tracking velocimetry (3D-PTV) is under current development (Nimmo Smith, 2008) and allows the measurement of three-dimensional distributions of all velocity components within a small sample volume. The applicability of in situ particle velocimetry techniques in the coastal ocean is limited by the cost of the instrumentation and its handling. It is thus important to note that more traditional methods such as freely rising or falling profilers equipped with shear-probes (Section 3.1) and ADCPs (Section 3.3) have been improved and extended in recent years. Specifically, the application of the variance method to high-resolution ADCP data allows automatic turbulence observations with moored equipment. Here, the cooperation of the instrument manufacturers with the researchers is an essential condition for advance of the methods. A fifth beam on commercial ADCPs would, for example, allow observation of TKE without the need for assumptions on turbulence anisotropy (Section 3.3).

Statistical turbulence modeling, which advanced rapidly in the 1970s (Section 4.2), has been strongly challenged during the last decade by the empirical KPP model (Sections 4.3 and 4.5). Empirical models can be easily extended to reproduce non-local processes such as free convection. Representing the same processes in statistical models requires retention of higher moments in the Friedman–Keller series, so that the models become computationally demanding. The capacity of statistical models to reproduce non-local phenomena has been impressively demonstrated by Canuto et al. (1994). A challenge now is formulation of computationally robust and economic statistical models to reproduce non-local features. The ultimate advantage of such models over empirical models should be their higher degree of generality, since statistical models are based on first principles. Three-dimensional models with relatively complex statistical turbulence closure have proliferated in recent years with the growth of computational capability (Section 4.5). It should be noted in this context that the turbulent mixing terms in Ocean General Circulation Models are the only terms about which there is a debate. Apart from these terms the models all use similar equations.

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